

# AGS Hong Kong Reinforced Steep Slopes (RSS)

Webinar – November 2023

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**TERRE ARMEE**

# INDEX

General introduction about Reinforced Steep Slopes

Design Principals and Engineering Fundamentals

Applications

Case Study Tindharia – Landslide protection and road widening

# General introduction about Reinforced Steep Slopes

**RSS - Environmental priority**

**Taking action for the climate**  
The solution allows to limit the future consequences of climate change. Lower carbon footprint

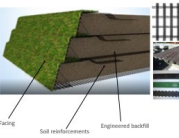
**Optimizing resources through the circular economy**  
The solution allows the use of site. Proper design and construction considerations can allow the use of marginal fills



**RSS is a reinforced soil structure**

**Reinforced soil structures** combine selected granular, engineered backfills with soil reinforcements with or without and a modular facing system.

This ideal combination creates a durable, mass gravity retaining soil bloc.




**RSS is a reinforced soil structure**

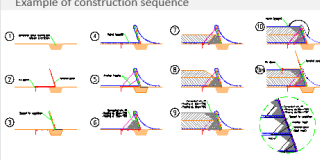
**Reinforced soil structures**

The choice and density of the soil reinforcement in a Reinforced soil structure is determined by:

- The static and dynamic design loads;
- The select backfill, which can have demanding mechanical and chemical properties;
- The site environmental conditions;
- Specific and potentially aggressive main-made restrictions: vibrations, pollution, fire ...



**Example of construction sequence**




**Reinforced steep slopes benefits**

**Adaptability**

RSS is a composite material coupled with a construction technique that is ideal for:

- Heterogeneous soils
- Unstable natural slopes
- Marginal foundations conditions
- Large settlement



**Reinforced Steep Slopes (RSS)**

An extension of MSE/SH technique  
Facing inclination from 45° - 1V:3H

2 main types: **Stone and Green finish**

| Facing  | Reinforcement  |
|---|--|
| <ul style="list-style-type: none"> <li>■ Stone</li> <li>■ Vegetated facing</li> </ul> | <ul style="list-style-type: none"> <li>■ Geogrids</li> <li>■ Geogrids</li> <li>■ Cellular confining systems</li> </ul> |



# RSS - Environmental priority



## **Taking action for the climate**

The solution allows to limit the future consequences of climate change. Lower carbon footprint



## **Optimizing resources through the circular economy**

The solution allows the use of site. Proper design and construction considerations can allow the use of marginal fills

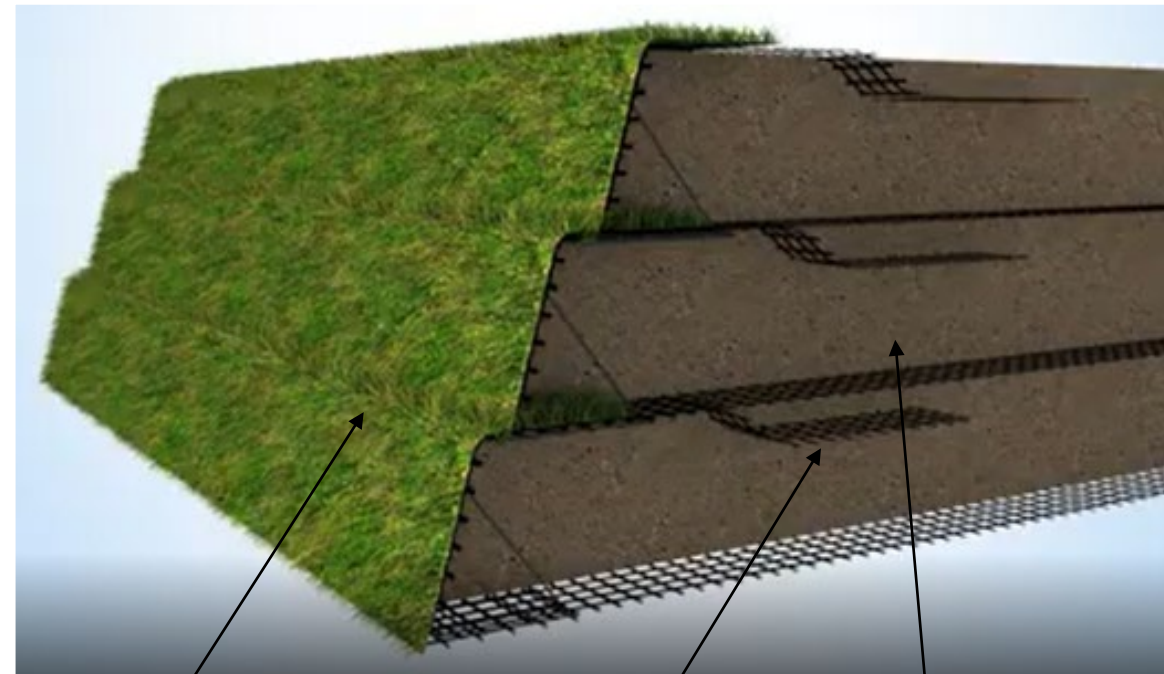


# RSS is a reinforced soil structure

## Reinforced soil structures

combine selected granular, engineered backfills with soil reinforcements with or without and a modular facing system.

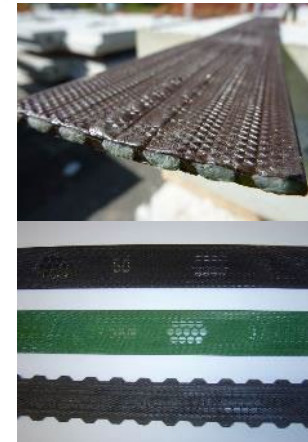
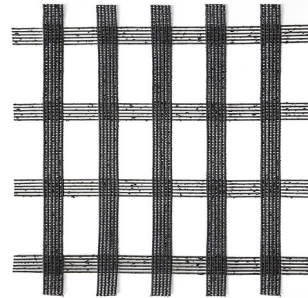
This ideal combination creates a durable, mass gravity retaining soil bloc.



Facing

Soil reinforcements

Engineered backfill

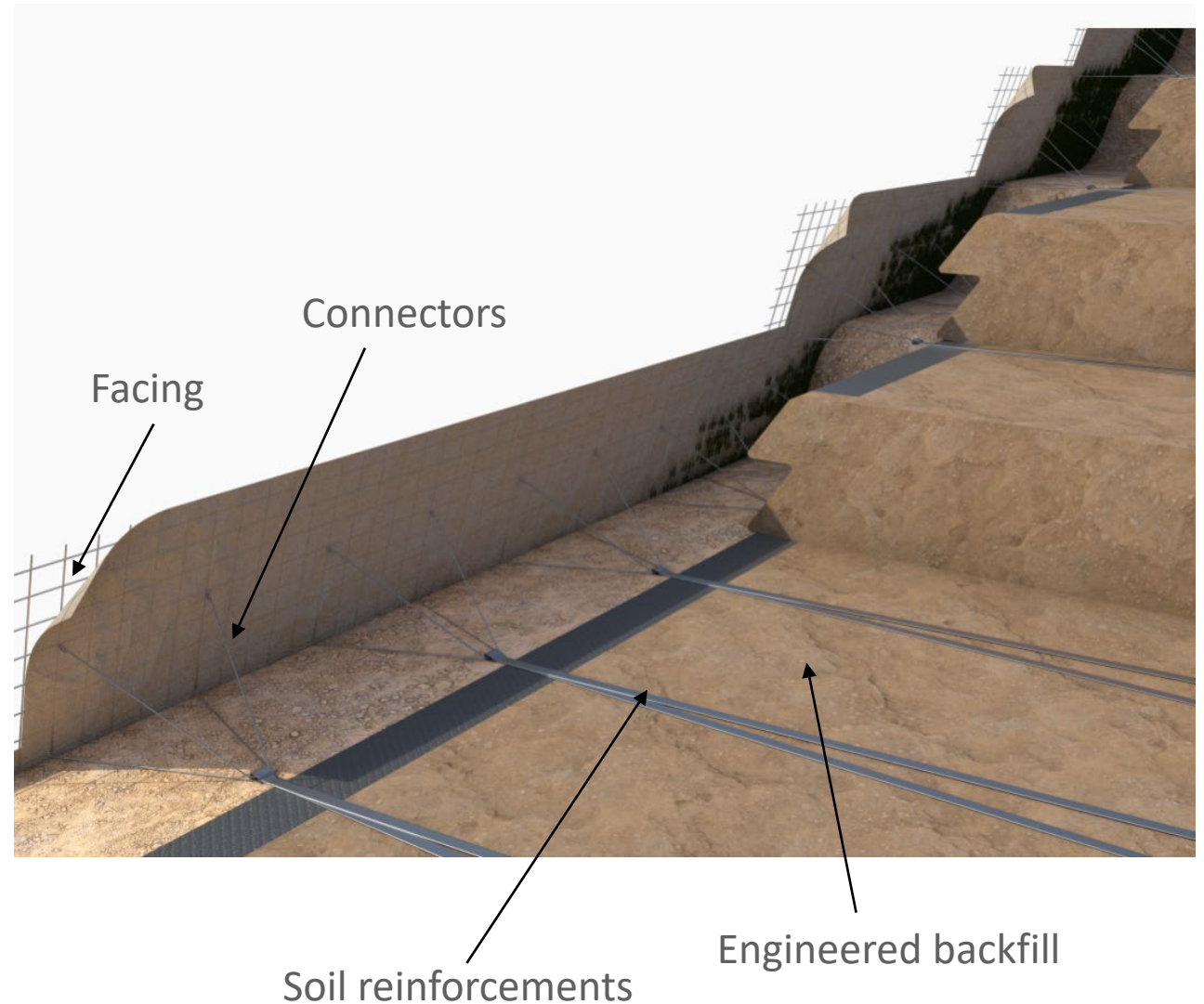


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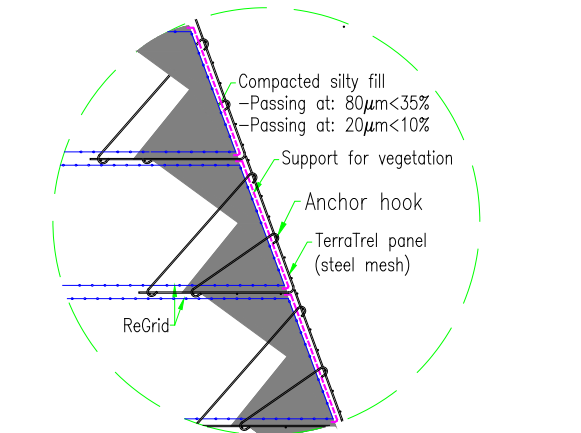
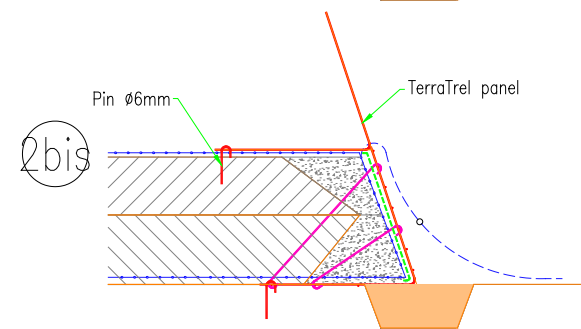
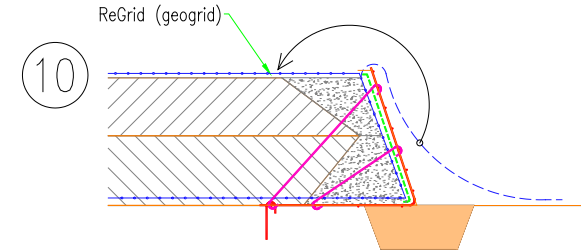
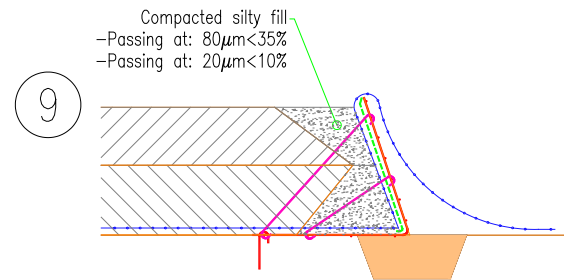
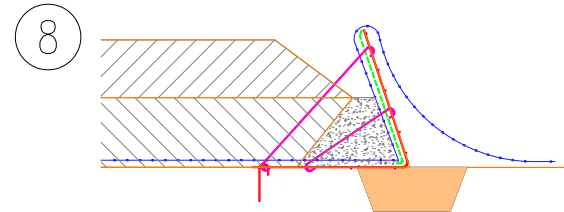
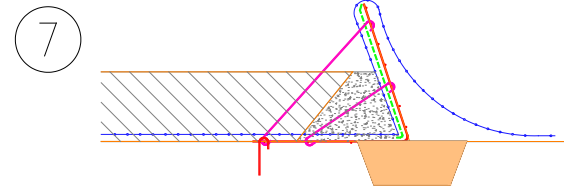
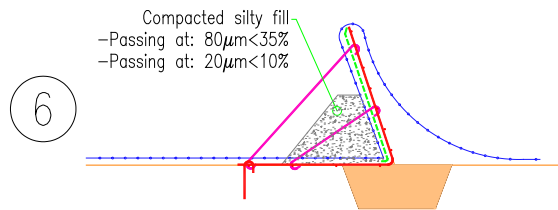
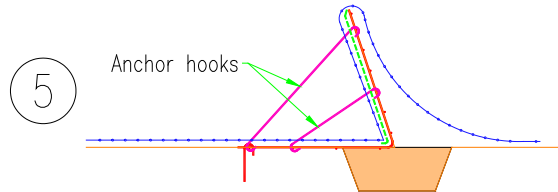
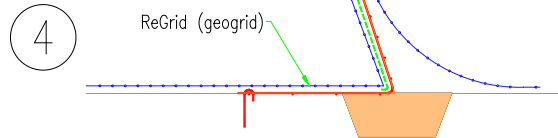
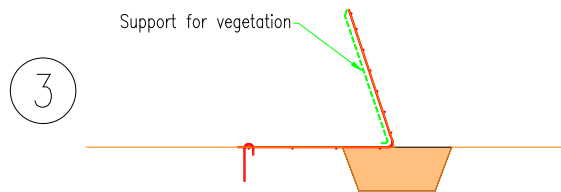
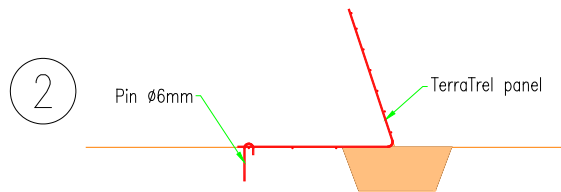
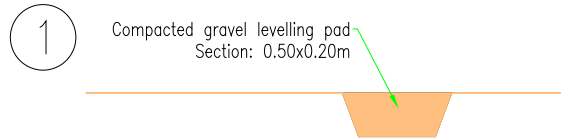
## Reinforced soil structures

The choice and density of the soil reinforcement in a Reinforced soil structure is determined by:

- The static and dynamic design loads;
- The select backfill, which can have demanding mechanical and chemical properties;
- The site environmental conditions;
- Specific and potentially aggressive man-made solicitations: vibrations, pollution, fire ...



# Example of construction sequence





# Reinforced steep slopes benefits

## Adaptability

RSS is a composite material coupled with a construction technique that is ideal for:

- Restricted right-of-way
- Unstable natural slopes
- Marginal foundation conditions
- Large settlement



### 1. Strength

- High load-bearing capacity

### 2. Reliability

- Up to 120 years durability

### 3. Resilience

- Effective absorption of vibrations (high speed trains, industrial equipment, explosion)
- Exceptional response to earthquakes

### 4. Flexibility

- Structures accept substantial total and differential settlement on poor foundations

### 5. Cost effectiveness

- Ease and speed of construction
- Economy of material
- Limited maintenance

### 6. Aesthetics

- Green or mineral finish

### 7. Environment

- Natural or recycled material
- Economy of material

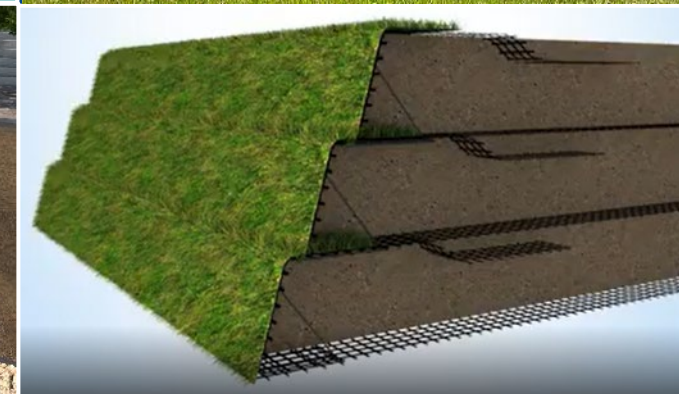


# Reinforced Steep Slopes (RSS)

An extension of MSEW technique  
Facing inclination from 45° - 1V:3H

2 main types: **Stone** and **Green finish**

| Finishing                        | Facing   | Reinforcement  |
|----------------------------------|--|--|
| <b>Green</b><br>Vegetated facing | <ul style="list-style-type: none"> <li>• Steel mesh</li> <li>• Wrap-around geogrids</li> <li>• Cellular confining systems</li> </ul> | <ul style="list-style-type: none"> <li>• <b>Geostrip</b></li> <li>• <b>Geogrids</b></li> </ul> |
| <b>Stone</b><br>Mineral facing   |  |  |



Mineral facing and geostrip reinforcement


Vegetated facing and geogrid reinforcement

# Design Principals and Engineering Fundamentals

- Structural approach

- Geotechnical approach

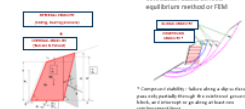
**Internal and surface drainage**



Internal drainage and surface water management at temporary or permanent stage are key

**Design verifications**

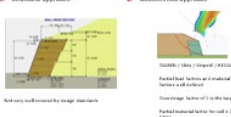
1. Structural or stress approach
2. Geotechnical approach - verification based on limit equilibrium method or FEM



\* Component stability: failure along a failure surface that penetrates through a structural element (pile, wall, or pier) or through an existing or future one (underpinning).

**Design verifications**

1. Structural approach
2. Geotechnical approach



Not every soil covered by design standards  
Partial load factors or partial factor combinations  
Over design factors of 1.5 to the target  
Partial resistance factor for soil is 0.5

**Design steps – Structural approach**

**GEOMETRY Definition**

- Substructure height
- Initial strip length definition
- Standard cross section
- Active zone definition

**EXTERNAL STABILITY**

- Sliding verification and bearing pressure

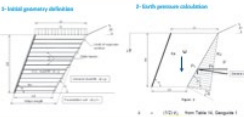
**INTERNAL STABILITY**

- Benthic rupture and global verifications



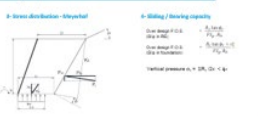
**Design steps**

1. Initial geometry definition
2. Earth pressure calculation



**Design steps**

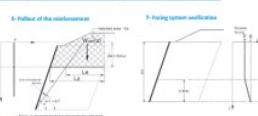
1. Stress distribution - slip surface
2. Sliding / bearing capacity



Vertical pressure  $q_v = \gamma \cdot z$   
Horizontal pressure  $q_h = \gamma \cdot z \cdot \tan^2(\alpha)$

**Design steps – Internal stability**

1. Failure of the reinforcement
2. Fixing system verification

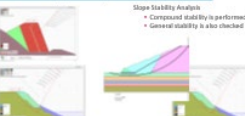


Angle  $\alpha = 20^\circ$  to  $45^\circ$  (maximum  $45^\circ$ )  
Failure  $\alpha = 20^\circ$  to  $45^\circ$  (maximum  $45^\circ$ )

**Geotechnical approach - Slope stability analysis**

Slope Stability Analysis

- Component stability is performed
- General stability is also checked



**Geotechnical approach – Slope stability analysis**

In assessing the stability of slopes, geotechnical engineers have to pay particular attention to **geology, drainage, groundwater, and the shear strength of the soils**.

The most common slope stability analysis methods are based on simplifying assumptions and **careful site investigation**. **Limit Equilibrium Methods** or FEM or FDM are used to examine slope stability.

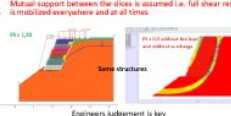
**Geotechnical approach – Slope stability analysis**

- Simple and yet applicable to complex problems
- Strength is assumed globally along a failure line
- Locally required strength, including connectors, is overloaded. It ignores local demand by "averaging" the load over all layers
- Alone it provides an important, but narrow, design perspective

→ **The structural approach is still key and will have to be done**

**Geotechnical approach – Slope stability analysis**

**Mutual support between the slices is assumed i.e. full shear resistance is mobilised everywhere and at all times**



Engineers judgement is key

**International standards**

| Standard  | Year | Scope                 |
|-----------|------|-----------------------|
| EN 1997-1 | 2004 | Design of foundations |
| EN 1997-2 | 2004 | Design of foundations |
| EN 1536   | 2008 | Design of foundations |
| EN 1537   | 2008 | Design of foundations |
| EN 1538   | 2008 | Design of foundations |
| EN 1539   | 2008 | Design of foundations |
| EN 1540   | 2008 | Design of foundations |
| EN 1541   | 2008 | Design of foundations |
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| EN 1566   | 2008 | Design of foundations |
| EN 1567   | 2008 | Design of foundations |
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| EN 1596   | 2008 | Design of foundations |
| EN 1597   | 2008 | Design of foundations |
| EN 1598   | 2008 | Design of foundations |
| EN 1599   | 2008 | Design of foundations |
| EN 1600   | 2008 | Design of foundations |

**Durability**

**Green Book 1:**

- Maintenance of the vegetation is key to protect against UV exposure
- Galvanized steel mesh is a good practice

**Stone tracks:**

- Galvanized steel mesh is mandatory
- Strength needs to be placed behind the stones to avoid UV exposure
- Polymeric Slings with mechanical connector is protected from UV exposure

**Net elements:**

- GEO verification to be provided
- Angular splices significantly affect the installation damage factor (lowers/limits strength)
- Trench soil FEM will have to comply with the net element limitations requirements

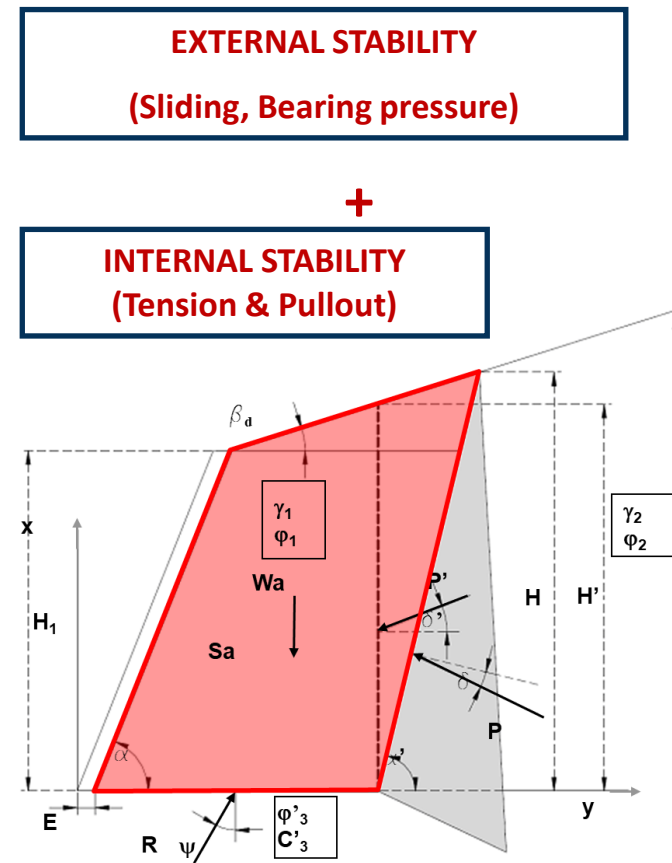
# Internal and surface drainage



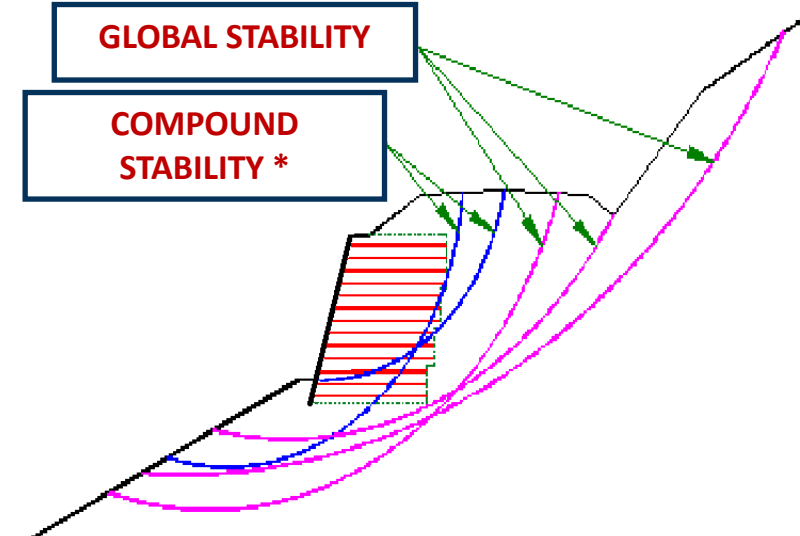
Internal drainage and surface water management at temporary or permanent stage are key

# Design verifications

## 1. Structural or stress approach



## 2. Geotechnical approach - Verification based on limit equilibrium method or FEM

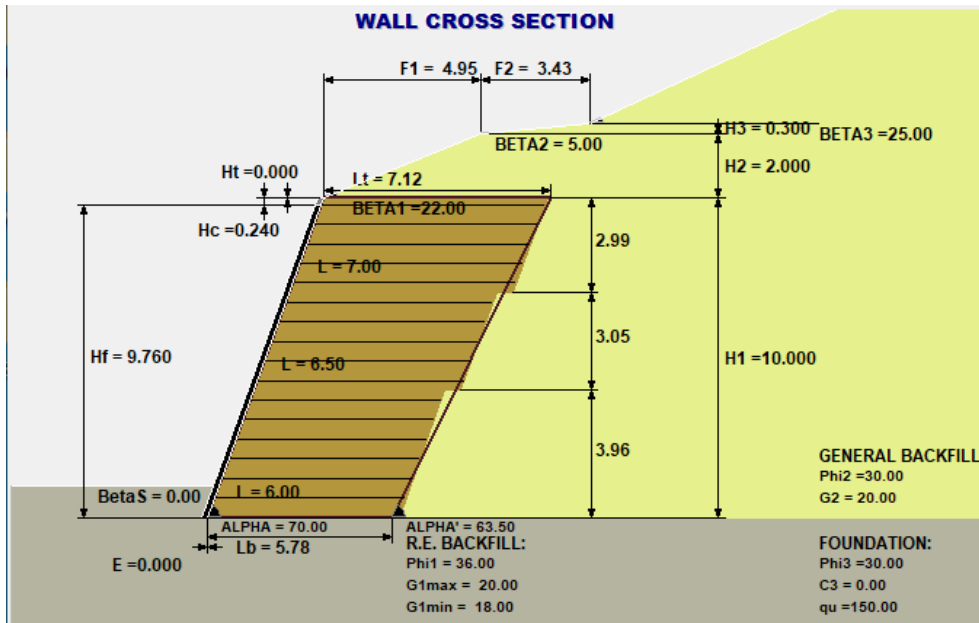


\* Compound stability : failure along a slip surface that pass only partially through the reinforced ground block, and intercept or go along at least one reinforcement layer



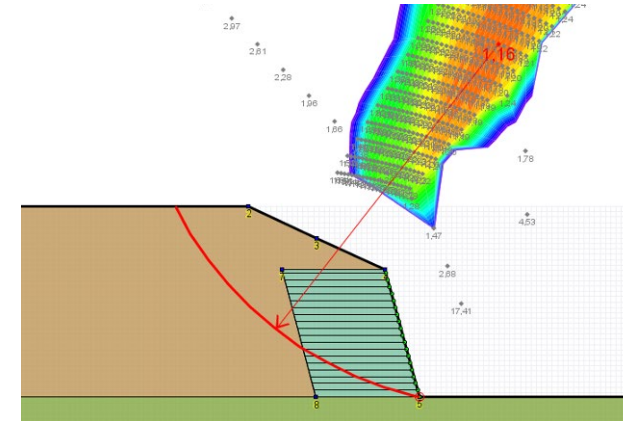
# Design verifications

## 1. Structural approach



Not very well covered by design standards

## 2. Geotechnical approach



TALREN / Slide / SlopeW / RESSA+

Partial load factors and material factors well defined

Overdesign factor of 1 is the target

Partial material factor for soil = 1.2  
 HKG6

# Design steps – Structural approach

## GEOMETRY Definition

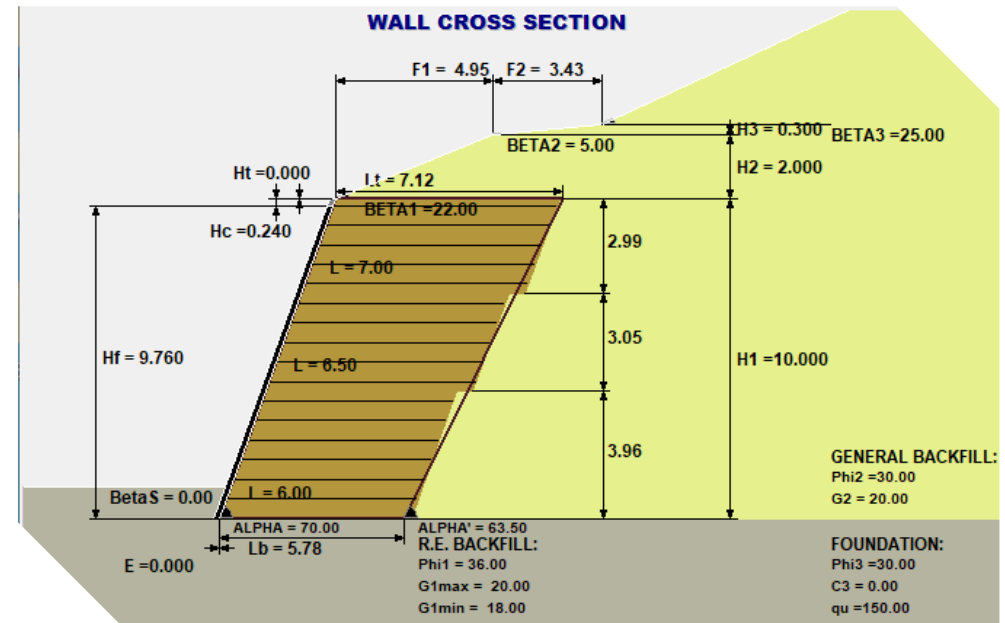
- Mechanical height
- **Initial strip length definition**
- Equivalent cross section
- Active zone definition

## EXTERNAL STABILITY

- **Sliding verification and bearing pressure**

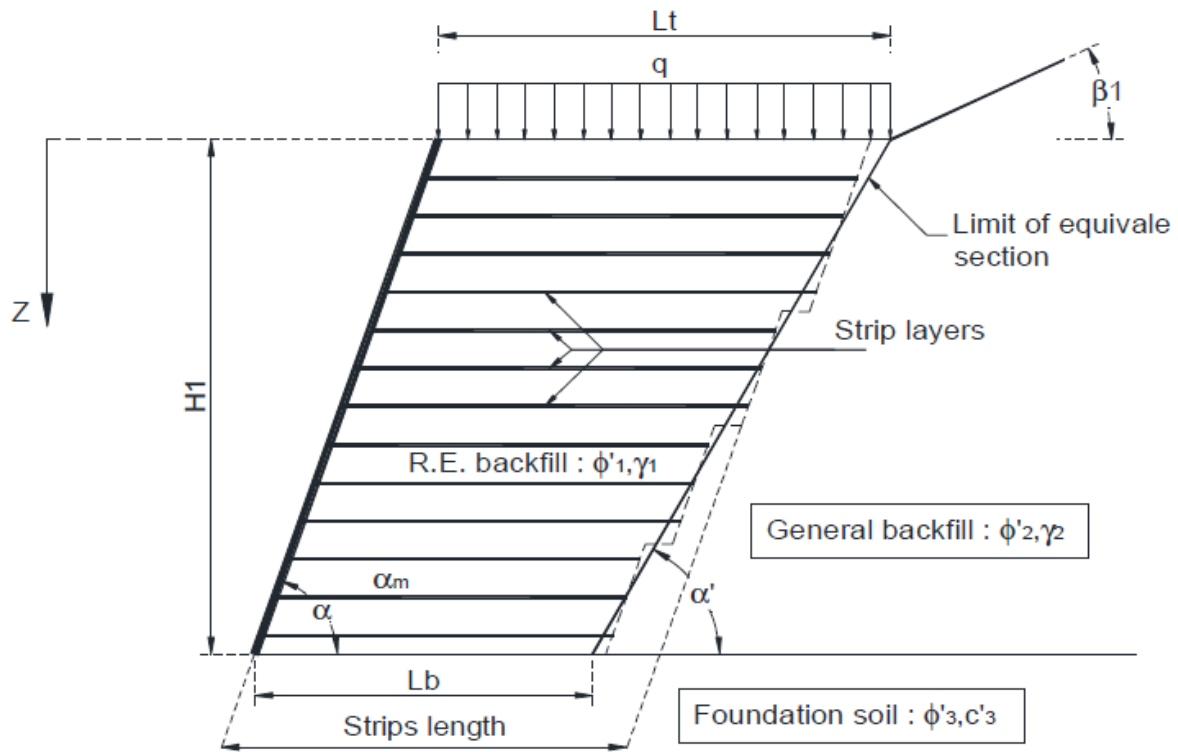
## INTERNAL STABILITY

- **Tensile rupture and pullout verification**



# Design steps

## 1- Initial geometry definition



## 2- Earth pressure calculation

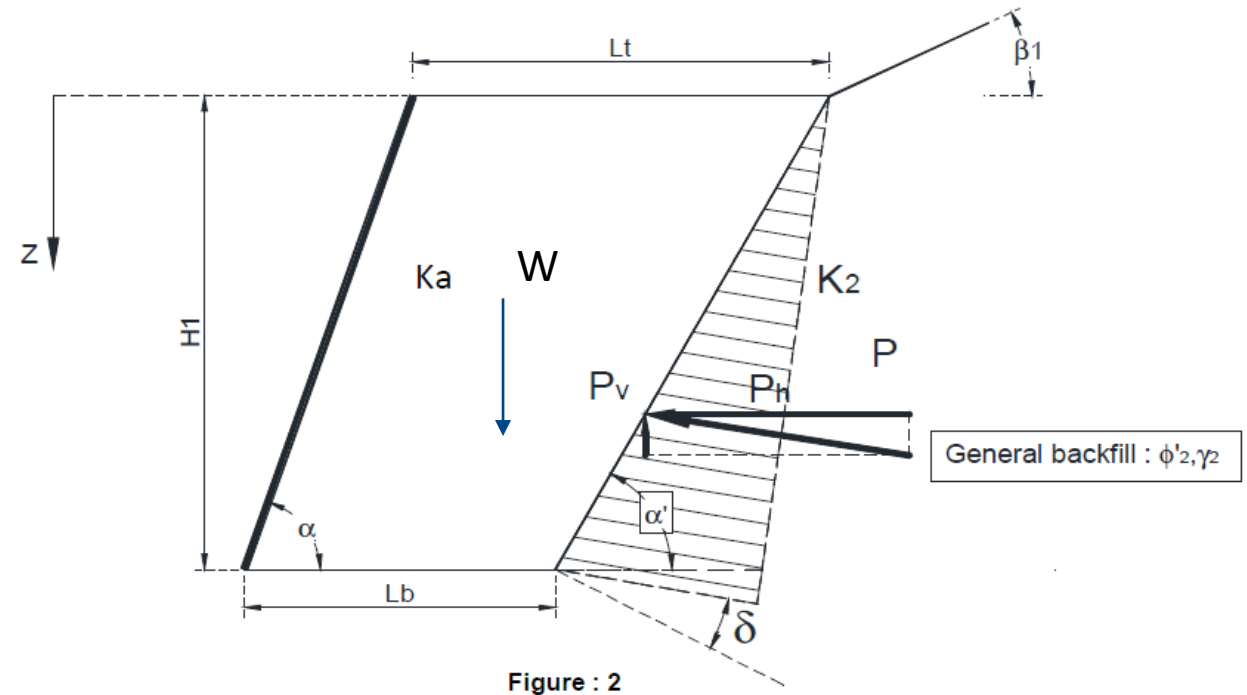
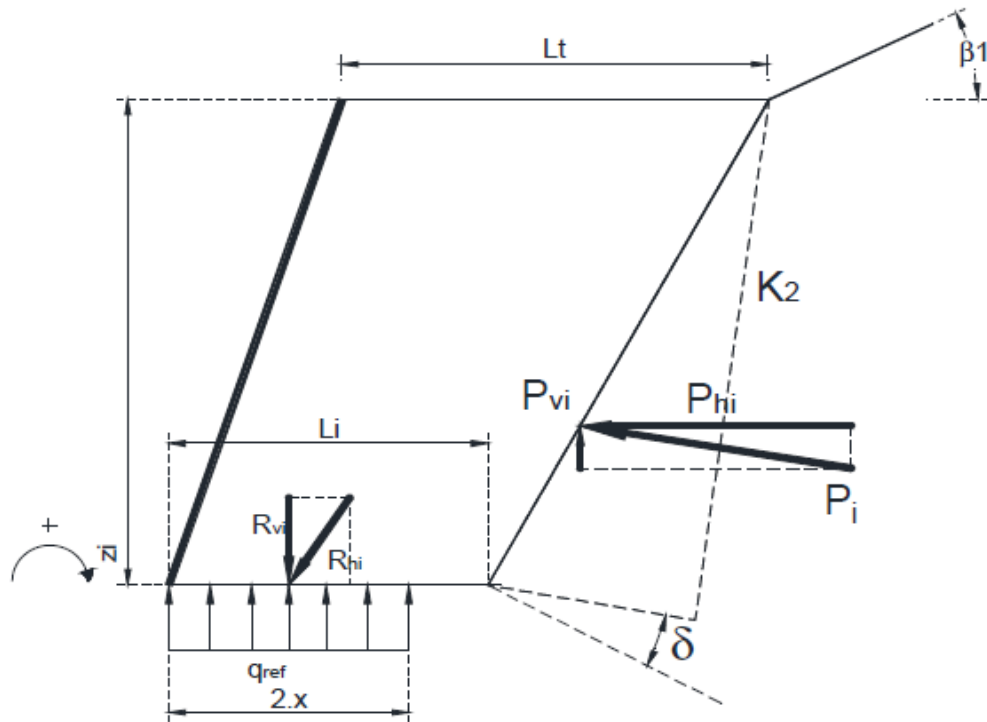


Figure : 2

$$\delta = \frac{1}{2} \phi'_2 \text{ from Table 14, Geoguide 1}$$

# Design steps

## 3- Stress distribution - Meyerhof



## 4- Sliding / Bearing capacity

Over design F.O.S. (Slip in RE) =  $\frac{R_v \tan \phi_r}{FS_g \cdot R_h}$

Over design F.O.S. (Slip in foundation) =  $\frac{R_v \tan \phi_r + cL}{FS_g \cdot R_h}$

Vertical pressure  $\sigma_v = \Sigma R_v / 2x < q_d$

# Design steps – Internal stability

## 5- Pullout of the reinforcement

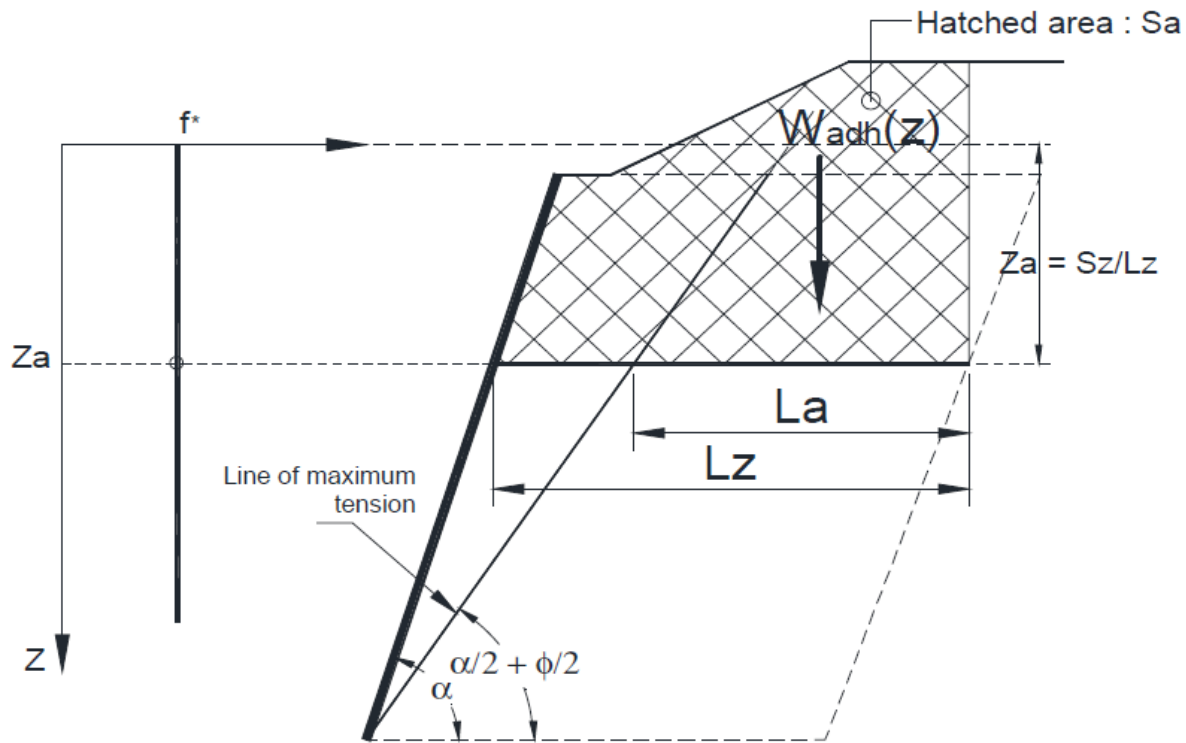
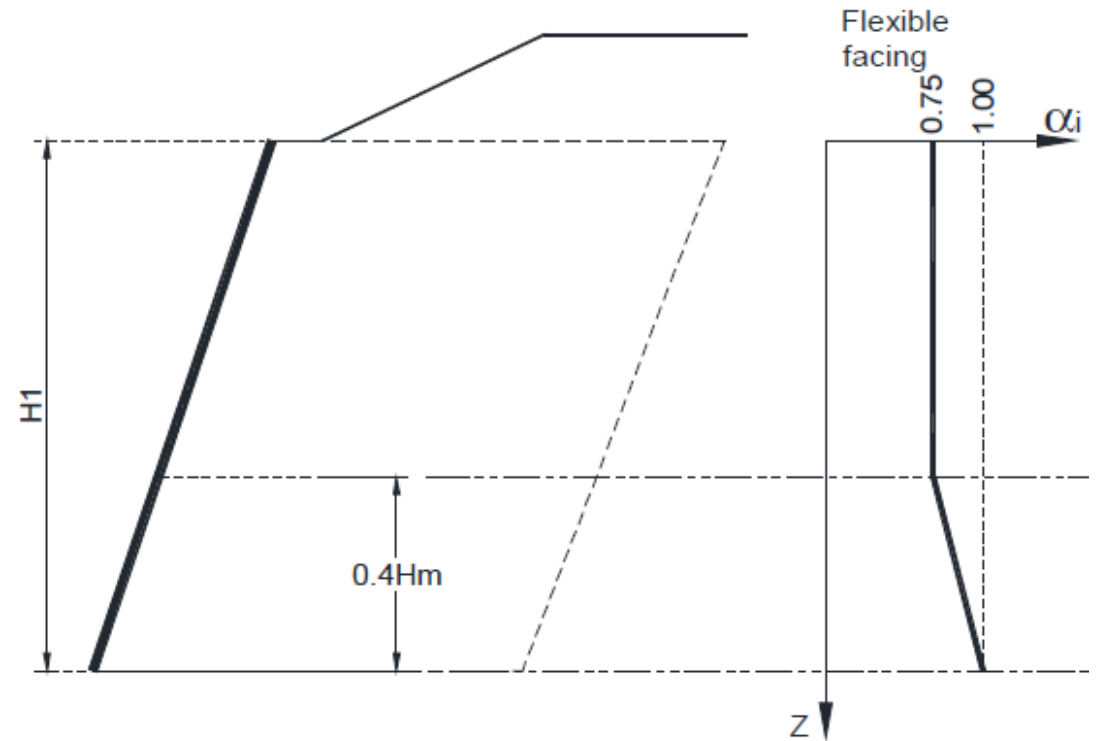


Figure - 7 : Anchored length and apparent friction with depth

## 7- Facing system verification



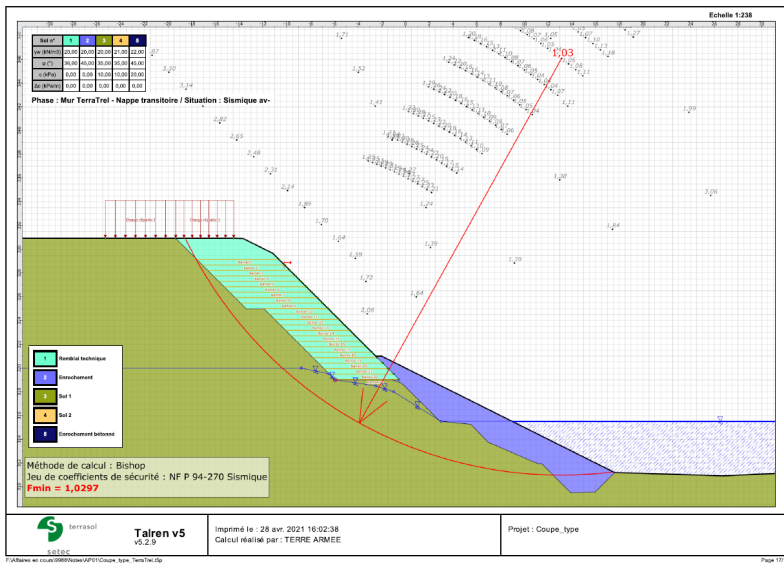
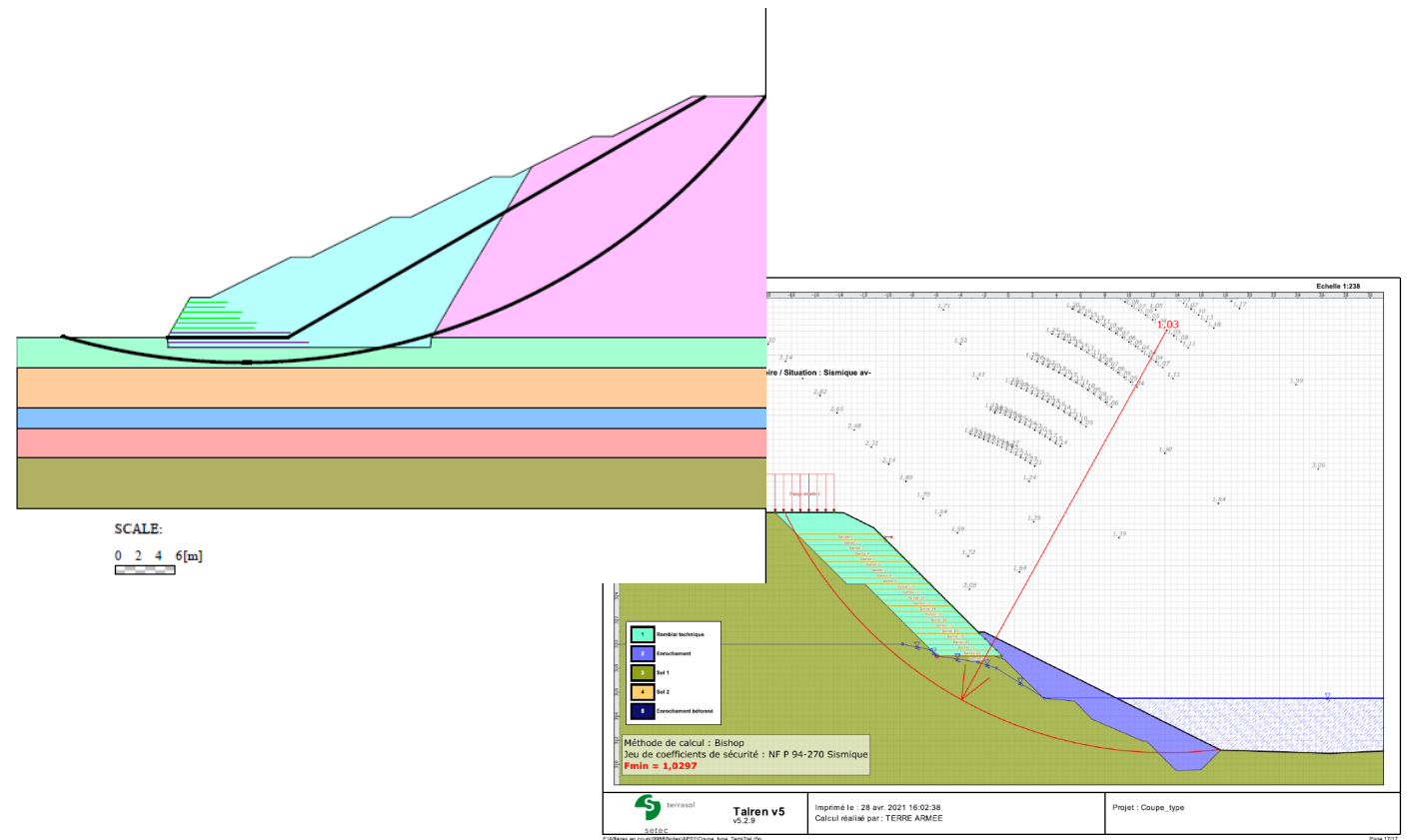
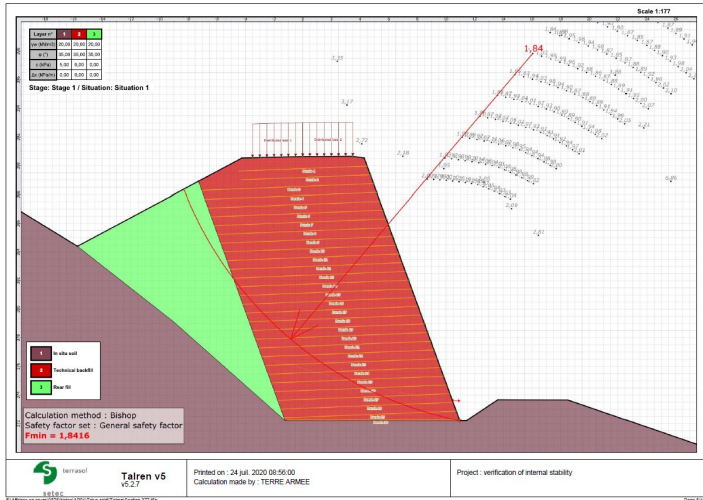
$$R_{f;di} = 2 \times b \times L_{effective} \times \mu^* \times \sigma_v(z) \times RF \geq T_{Max}$$

$$R_{facing} \geq T_{Max-facing} = T_{Max} \times \alpha_i$$

# Geotechnical approach - Slope stability analysis

## Slope Stability Analysis

- Compound stability is performed
- General stability is also checked



# Geotechnical approach – Slope stability analysis

In assessing the stability of slopes, geotechnical engineers have to pay particular attention to **geology, drainage, groundwater**, and the **shear strength** of the soils.

The most common slope stability analysis methods are based on simplifying assumptions and **careful site investigation**. **Limit Equilibrium Methods** or FEM or FDM are used to examine slope stability

# Geotechnical approach – Slope stability analysis

- Simple and yet applicable to complex problems
- Strength is examined globally along a failure line
- Locally required strength , including connections, is overlooked. It ignores local demand by “averaging” the load over all layers
- Alone it provides an important, but narrow, design perspective

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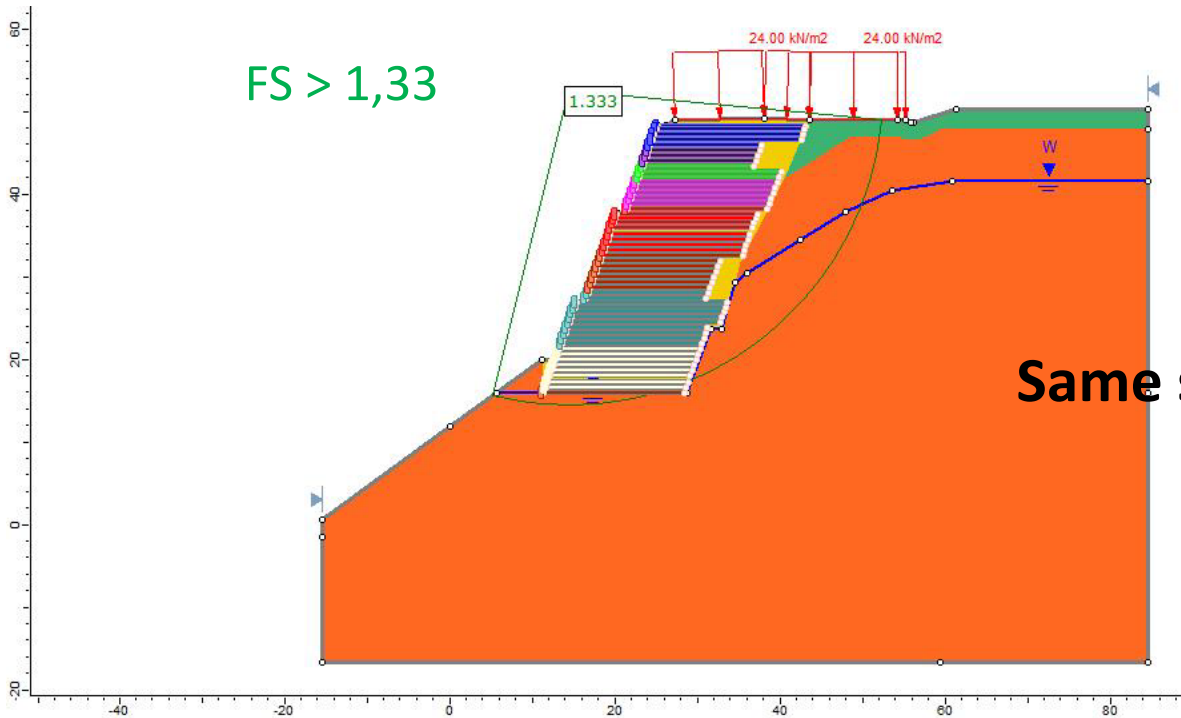


# Geotechnical approach – Slope stability analysis

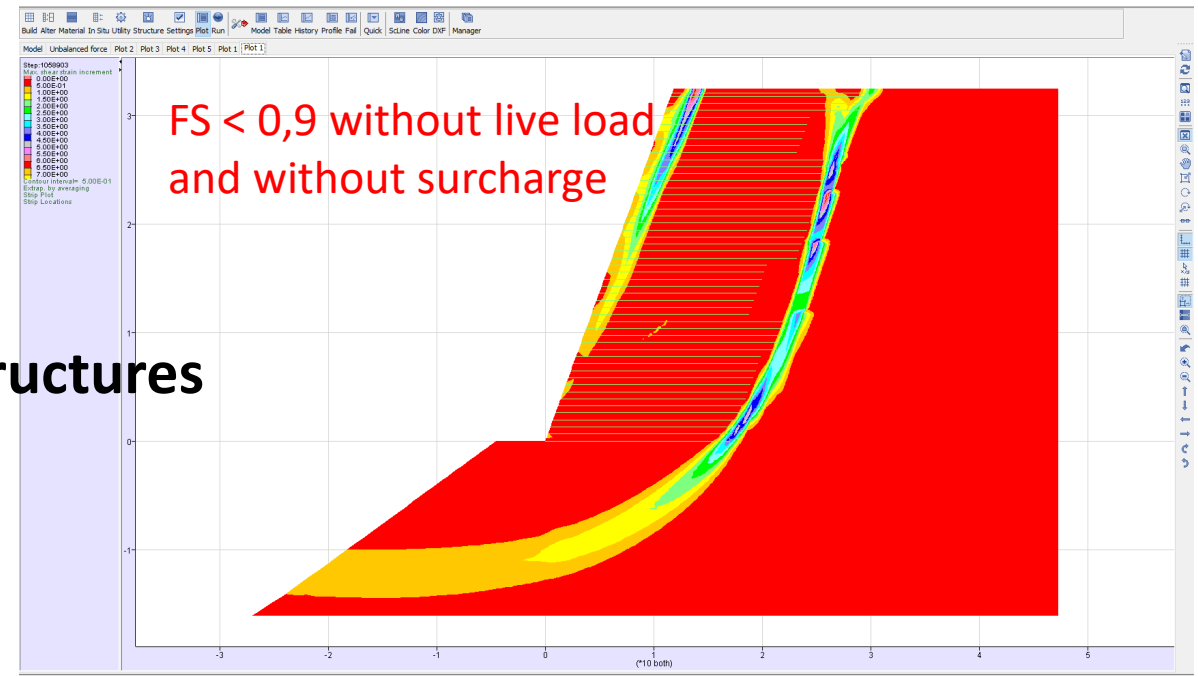


Mutual support between the slices is assumed i.e. full shear resistance is mobilized everywhere and at all times

FS > 1,33



Same structures



FS < 0,9 without live load and without surcharge

Engineers judgement is key

# International standards

| FILL TYPE     |   | Type 1                       | Type 2   |                               | Type 3       |           | Type 4 |        |
|---------------|---|------------------------------|----------|-------------------------------|--------------|-----------|--------|--------|
|               |   | Draining                     | Granular |                               | Intermediate |           | Fine   |        |
|               | Geomechanical characteristics                                       | % passing the 80micron sieve | <5%      | <12%                          | 12 to 35%    | 12 to 35% | >35%   | Others |
|               |   | % passing the 20micron sieve | n.a.     | n.a.                          | <10%         | >10%      | <40%   |        |
|               |   | Plasticity Index             | n.a.     | n.a.                          | n.a.         | <25       | <25    |        |
| APPLICATION   |   |                              |          |                               |              |           |        |        |
|               | Parts of structure exposed to flooding and/or rapid water draw-down | A                            | B        | B                             | D            | D         | D      |        |
|               | Structure supporting bridge abutments, railways, buildings          | A                            | A        | B                             | C (a)        | D         | D      |        |
|               | High reinforced fill walls  | A                            | A        | B                             | B            | D         | D      |        |
|               | High reinforced fill slopes   | A                            | A        | B                             | B            | C (b)     | C (b)  |        |
|               | Common walls and slopes   | A                            | A        | A                             | B            | C (c)     | C (c)  |        |
| REINFORCEMENT |   |                              |          |                               |              |           |        |        |
|               | Smooth strips or rods (metallic or polymeric)                       | A                            | A        |                               | C (d)        |           | D      |        |
|               | Ribbed strips or rods, ladders (metallic or polymeric)              | A                            | A        |                               | B            | C (d)     | D      |        |
|               | Bar mats, ladders, meshes, grids, sheets (metallic or polymeric)    | A                            | A        |                               | B            | C (d)     | D      |        |
|               | Draining geosynthetics (in-plane permeability)                      | B                            | A        |                               | A            |           | C (b)  |        |
| FACING        |   |                              |          |                               |              |           |        |        |
|               | Rigid   | A                            | A        |                               | D(a)         |           | D      |        |
|               | Semi flexible   | A                            | A        |                               | C(e)         |           | D      |        |
|               | Flexible  | A                            | A        |                               | A            | B         | C(e)   |        |
|               |   | KEY :                        |          | A = Often Used                |              |           |        |        |
|               |   |                              |          | B = Sometimes Used            |              |           |        |        |
|               |   |                              |          | C = Subject to Specific Study |              |           |        |        |
|               |   |                              |          | D = Not Recommended           |              |           |        |        |

**BS 8006: 2010**

**European code : EN 14475**

Execution of special geotechnical Works - Reinforced fill

With an increasing fine content the limitations increases

## NOTES

**General** The typical combinations above are given for general guidance only and are not intended to be a specification of where various fills or components may be used. The brief descriptions of the fills above are only some of the principle characteristics and do not fully describe a fill. The design documents or a project should specify the particular fills and components which should be used. Fine fill which is too wet of optimum is difficult to compact and likely to cause facings, if used, to go out of alignment during compaction. Fine fill laid and compacted in adverse weather conditions may be problematic. Frost susceptibility should be checked if applied in cold climates.

## Specific

- a If adequate compaction is not achieved then differential settlements between facing and reinforcements may occur which may overload the connection.
- b The effect of the drainage properties on the fill characteristics should be assessed.
- c Special attention should be paid to : angle of internal friction, compaction procedure with respect to moisture content and climatic conditions, need for drainage layers.
- d The fill-reinforcement interaction should be assessed for long term and during construction conditions
- e Special attention should be paid to the control of the alignment of the facing units (if any) during construction.

# Durability

## Green finish :

- Maintenance of the vegetation is key to protect against UV exposure
- Galvanized steel mesh is a good practice

## Stone finish

- Galvanized steel mesh is mandatory
- Geogrids needs to be placed behind the stones to avoid UV exposure
- Polymeric Strap with mechanical connector is protected from UV exposure

## Reinforcements

- GEO certification to be provided
- Angular soil can significantly affect the installation damage factor (woven/knitted geogrids)
- Technical fill will have to comply with the reinforcement limitations requirements

# Applications

**RSS: Application**

- Retaining structures
- Slope stabilization
- Slopes
- Roadside ditches
- Hazardous waste perimeter control
- Slope protection (steep, water, ...)
- Urban Access lanes
- Landfill Mining / Tunnel & pipe installation



**References – Green finish**



- Saint Denis (France)
- Commenet park (France)
- Imperial slope (Spain)
- France (St. Exupéry Field) (France)

**References**



- Approved road
- Road work (France)
- Z-Morph tunnel (India)
- Shoreline road (France)

**Z-Morph tunnel (India) – Stone finish**



- Stone finish
- Galvanized steel mesh
- Galvanized corrugated Geogrid

**Solan Kathlighat (India) – Green finish**



- Clad walling
- Green slope
- Galvanized steel mesh
- Galvanized corrugated Geogrid

**Steep Slopes – Railway (Sumbang Yard)**




- Geogrid reinforcement
- Stone wall
- Galvanized steel mesh
- Galvanized corrugated Geogrid

**Steep Slopes – Railway (Rangoon Yard)**



- Safety yard platform
- Galvanized steel mesh
- Galvanized corrugated Geogrid

**Porto di Genova (Italy) – Geogrid**



- Green slope
- Galvanized steel mesh
- Galvanized corrugated Geogrid

**Les Ardennes (France) – Geogrids**



- Temporary wall
- Green slope
- Black steel mesh
- Black steel for the concrete Geogrids

**Application - Doho airport (Indonesia)**



- Applications : Runway
- Green slope
- Galvanized
- Wet pavement geogrids
- Ballings

**Application – Avalanche and Rockfall**



**Application - Landfill**





# RSS: Application

- Retaining structures
- Platform support
- Slope rehabilitation
- Slopes
- Embankments
- Gravitational risk protection (rockfall, slides,...)
- Industrial protection (blasts, noise, ...)
- Dykes, berms, levees
- Landfills, Mining / Tunnel dump structures





# References – Green finish



**Saint Véran (France)**



**Courneuve park (France)**



**Vegetated slope (Japan)**



**Roma G.R.A Quad (Italia)**



**A4 Horselgau Road (Germany)**



# References



Approach road

Tunnel muck  
dump structure

**Z-Morh tunnel (India)**



Tunnel platform  
support



Tunnel portal

**Mont Blanc tunnel (France)**



# Z-Morh tunnel (India) – Stone finish



Stone finish  
Galvanized steel mesh  
Galvanized connectors  
Geostrip





# Solan Kaithlighat (India) – Green finish



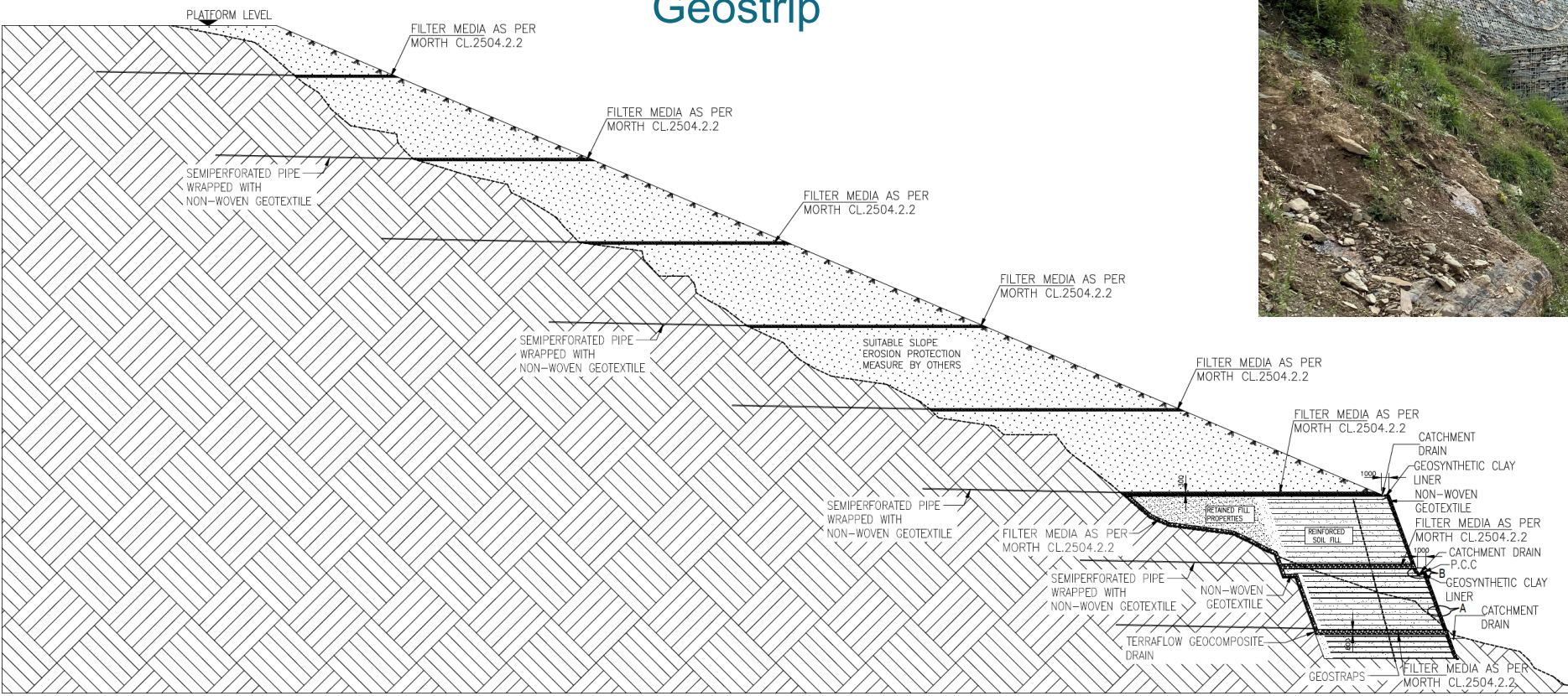
Road widening  
Green finish  
Galvanized steel mesh  
Galvanized connectors  
Geostrip





# Steep Slopes – Railways (Sumber Yard)

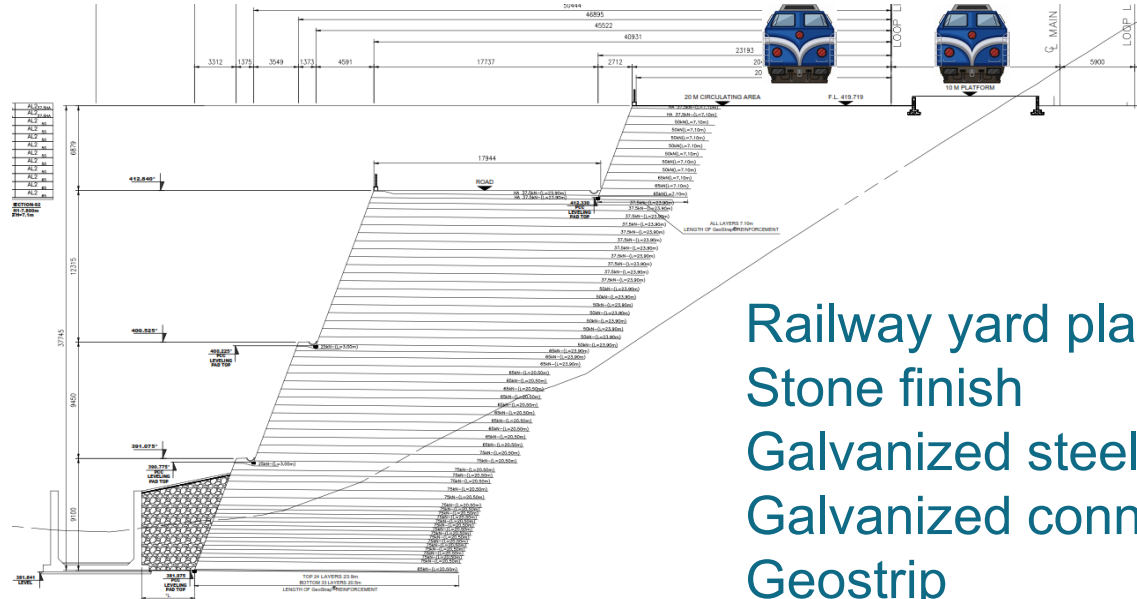
Slope stabilization  
 Stone finish  
 Galvanized steel mesh  
 Galvanized connectors  
 Geostrip



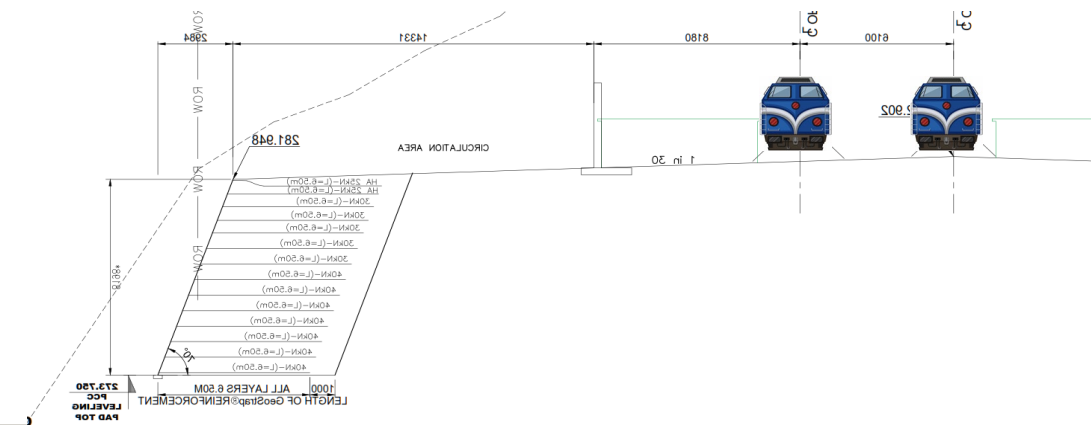
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# Steep Slopes – Railways (Rangpo Yard)



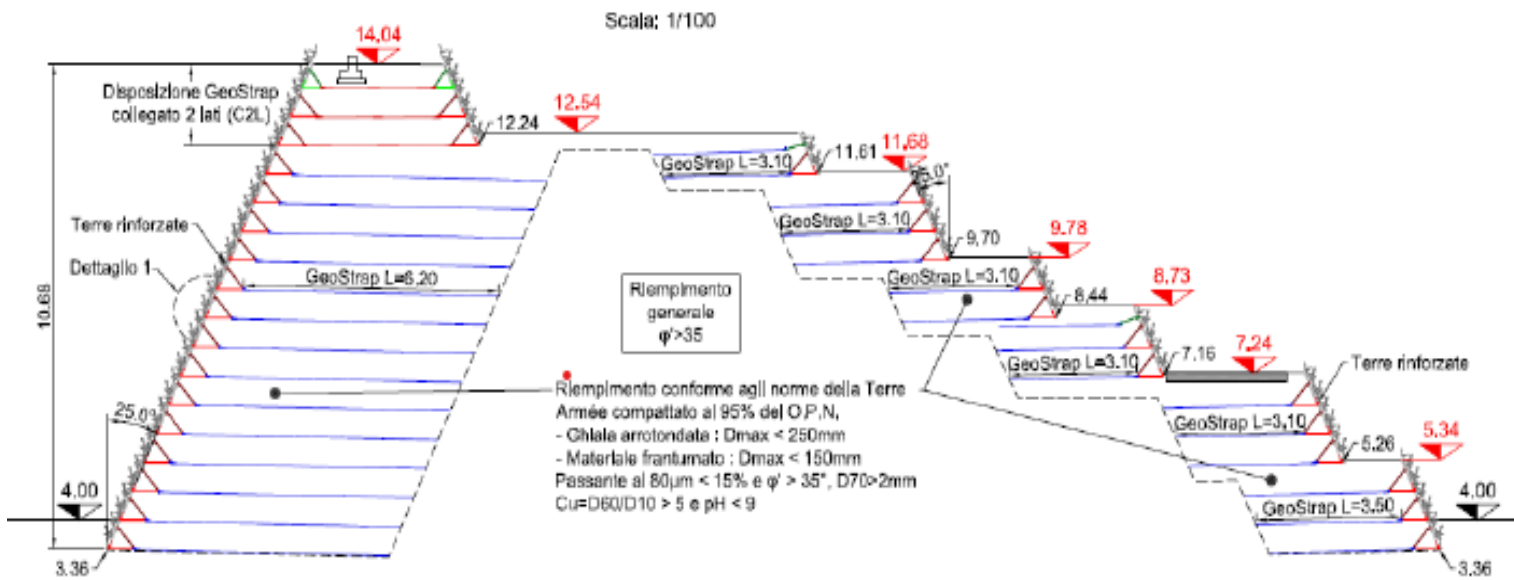
Railway yard platform  
 Stone finish  
 Galvanized steel mesh  
 Galvanized connectors  
 Geostrip





# Porto di Genova (Italia) - Geostrip

Green finish  
Galvanized steel mesh  
Galvanized connectors  
Geostrip





# Les Ardoines (France) - Geogrids

Temporary wall



Green finish  
Black steel mesh  
Black steel for the connectors  
Geogrids



# Application - Dhoho airport (Indonesia)

Application : Runway

Green finish

Solution :

- Wraparound geogrids
- Soil bags





# Application – Avalanche and Rockfall













# Application - Landfill





**Index**

|  |  |
|--|--|
| Location   |   |
| Site condition and problems                        |   |
| Objectives   |   |
| Solution   |   |
| Design   |   |
| Execution  |  |
| Completed Structure                                |   |
| Benefits - Backfill Reduction & Water Conservation |  |

38

# Index

## Location



## Site condition and problems



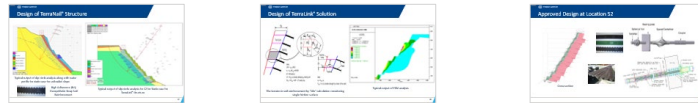
## Objectives



## Solution



## Design



## Execution



## Completed Structure



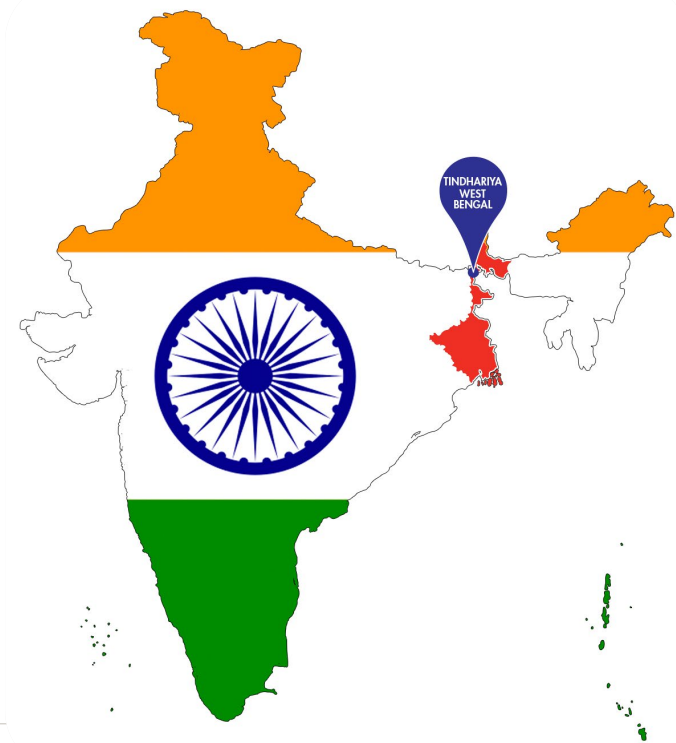
## Benefits - Backfill Reduction & Water Conservation





# Landslide in 2011 at Tindharia, West Bengal (INDIA) – UNESCO World Heritage Site

- Earthquake of **magnitude 6.9 ( $M_w$ )** followed by heavy rainfall.
- Massive landslide occurred resulted in collapse of a section of National Highway (NH) - 110 (previously NH-55).
- **Severe socio-economic impact.**





# Tindharia

Railway settlement town between Siliguri and Darjeeling in West Bengal, India



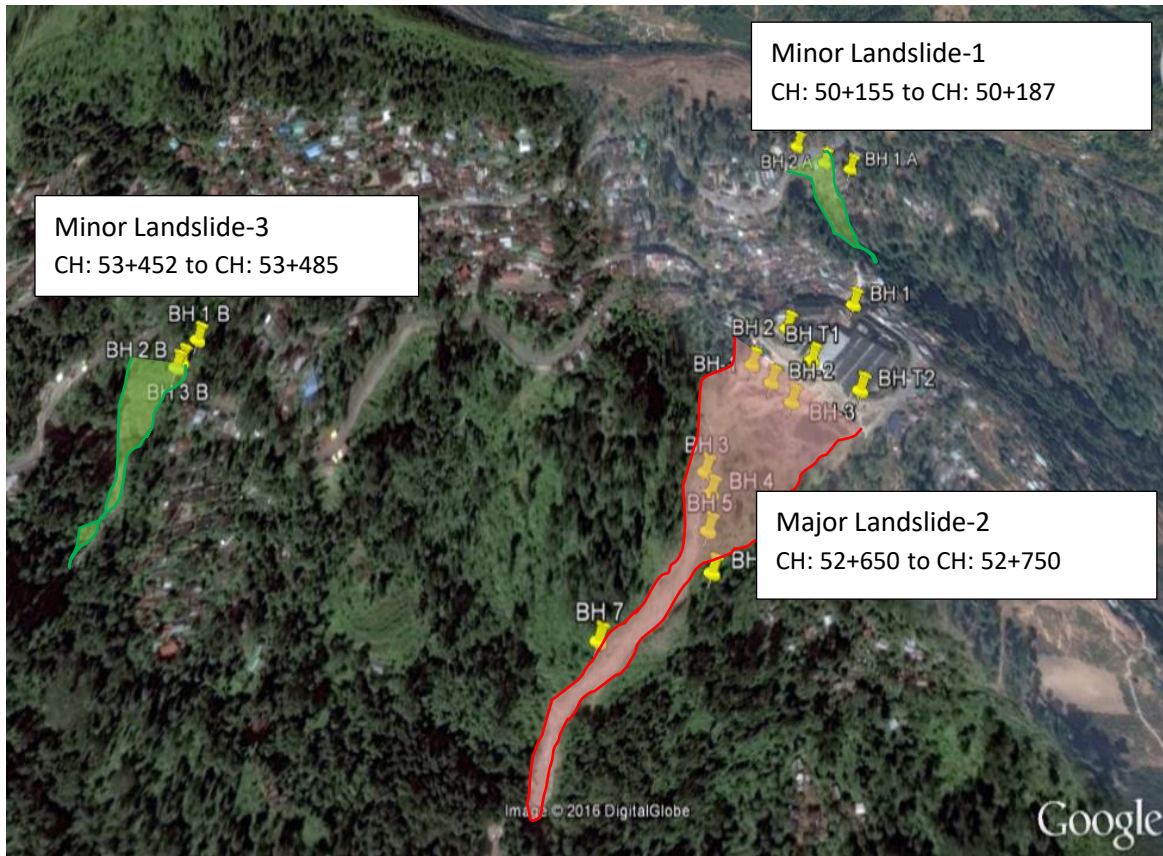
September 2011

massive landslides following a combination of an earthquake and heavy rains



# Site Conditions and Problems

## Three landslide Locations



Retention height of three locations varied from 34.9 m to 102.8 m.

**Non-availability of Backfill Material**

Space Constraint for Foundation Base Width for widening

**Safety Challenge Working at such Height**

Unfavourable Geology

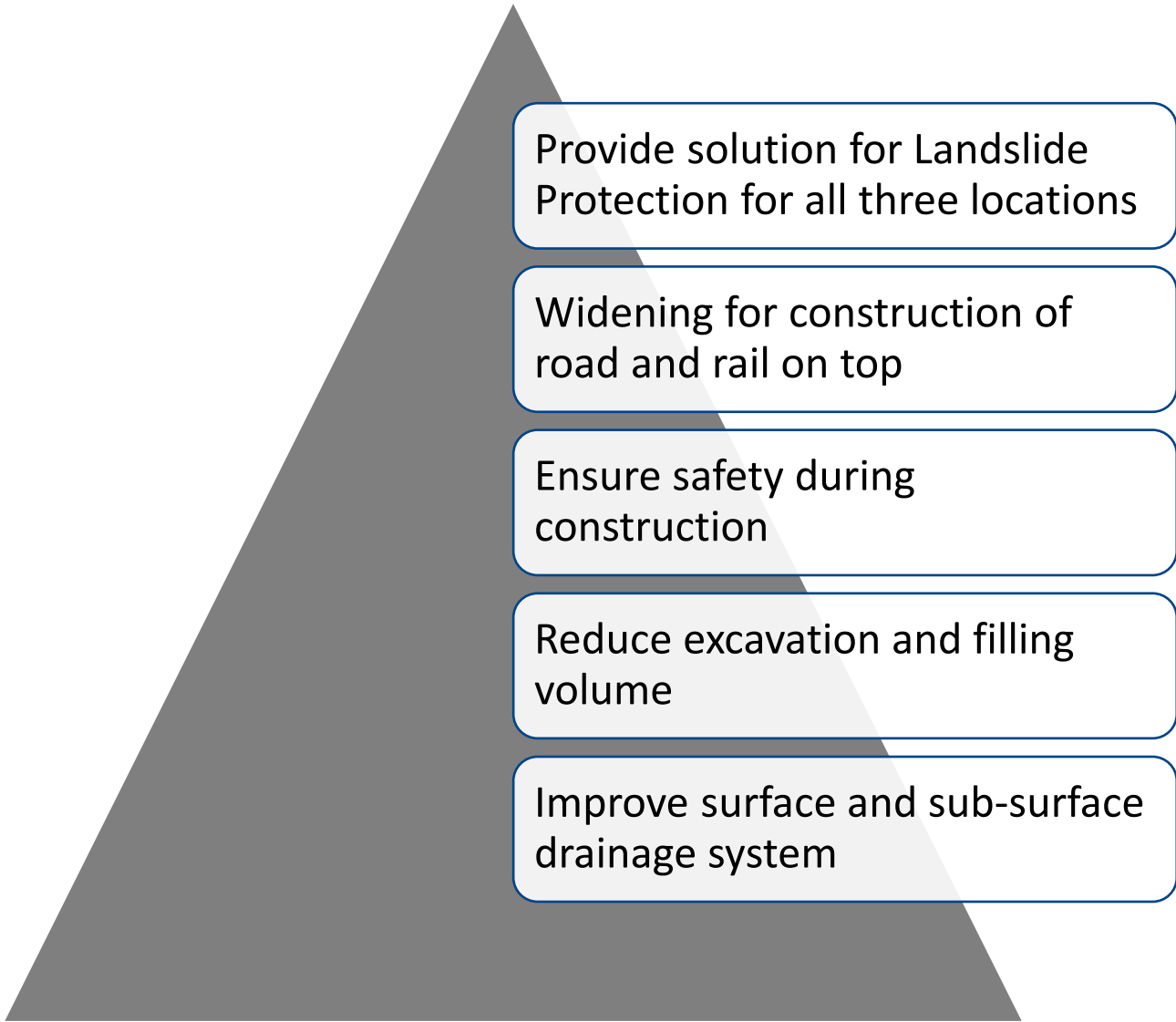
**Inclement Weather**

Presence of Ground Water

**Timely Completion – Impact of Monsoon and long labour Strike**

Impact of COVID-19

# Objectives



Provide solution for Landslide Protection for all three locations

Widening for construction of road and rail on top

Ensure safety during construction

Reduce excavation and filling volume

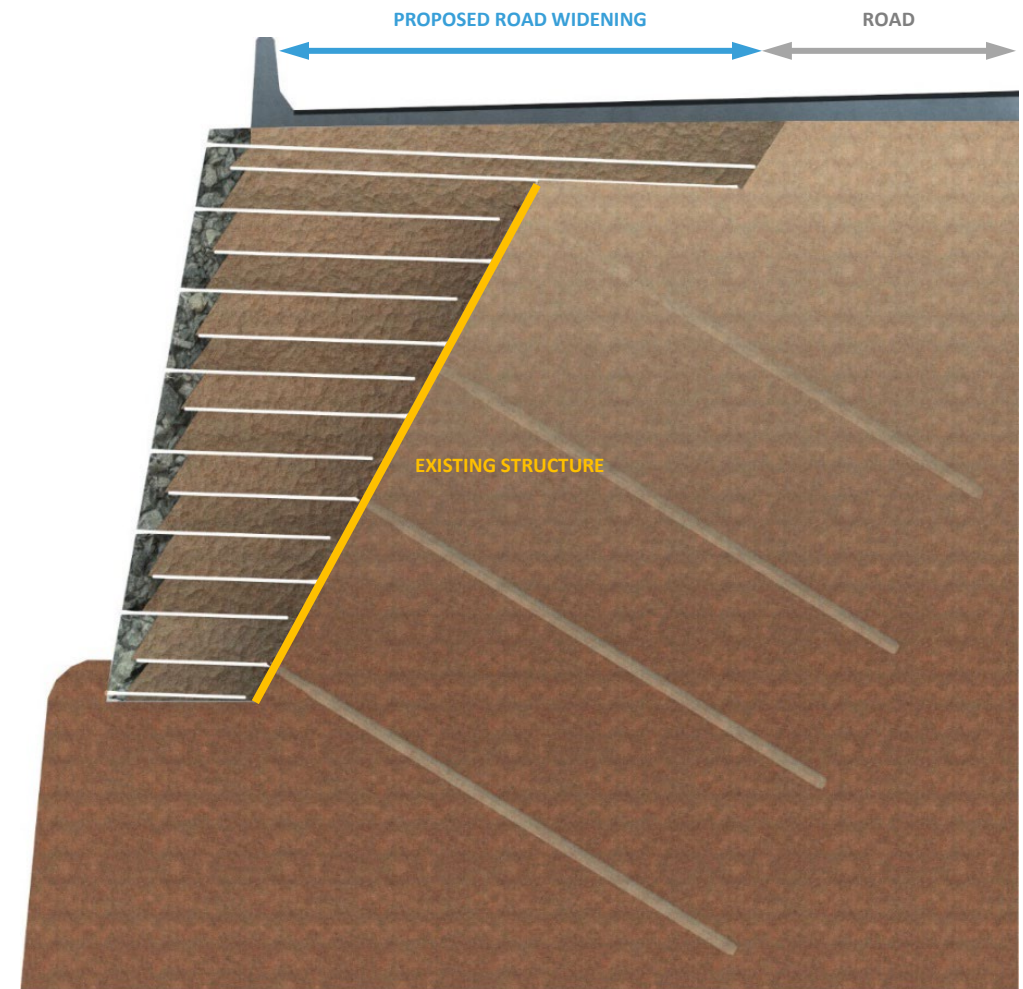
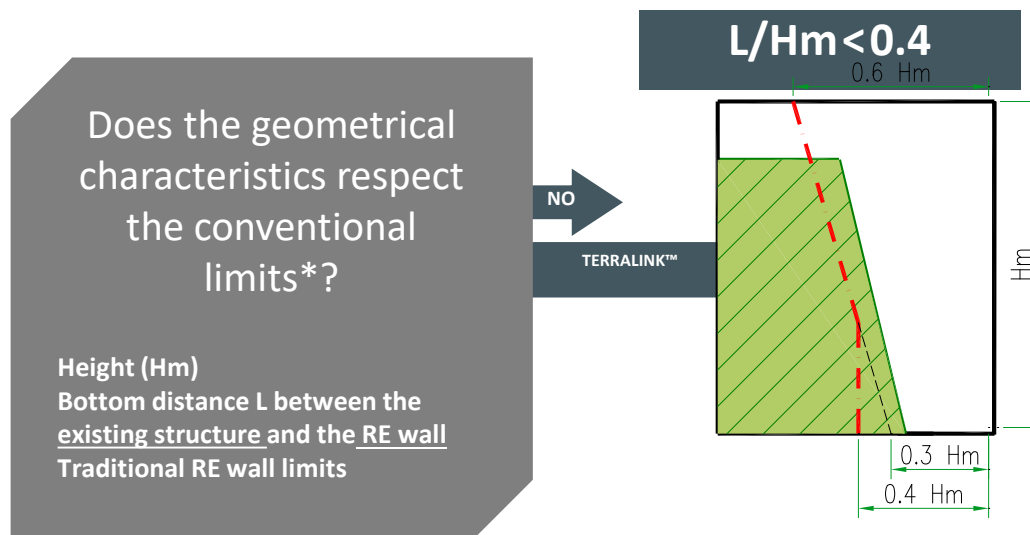
Improve surface and sub-surface drainage system



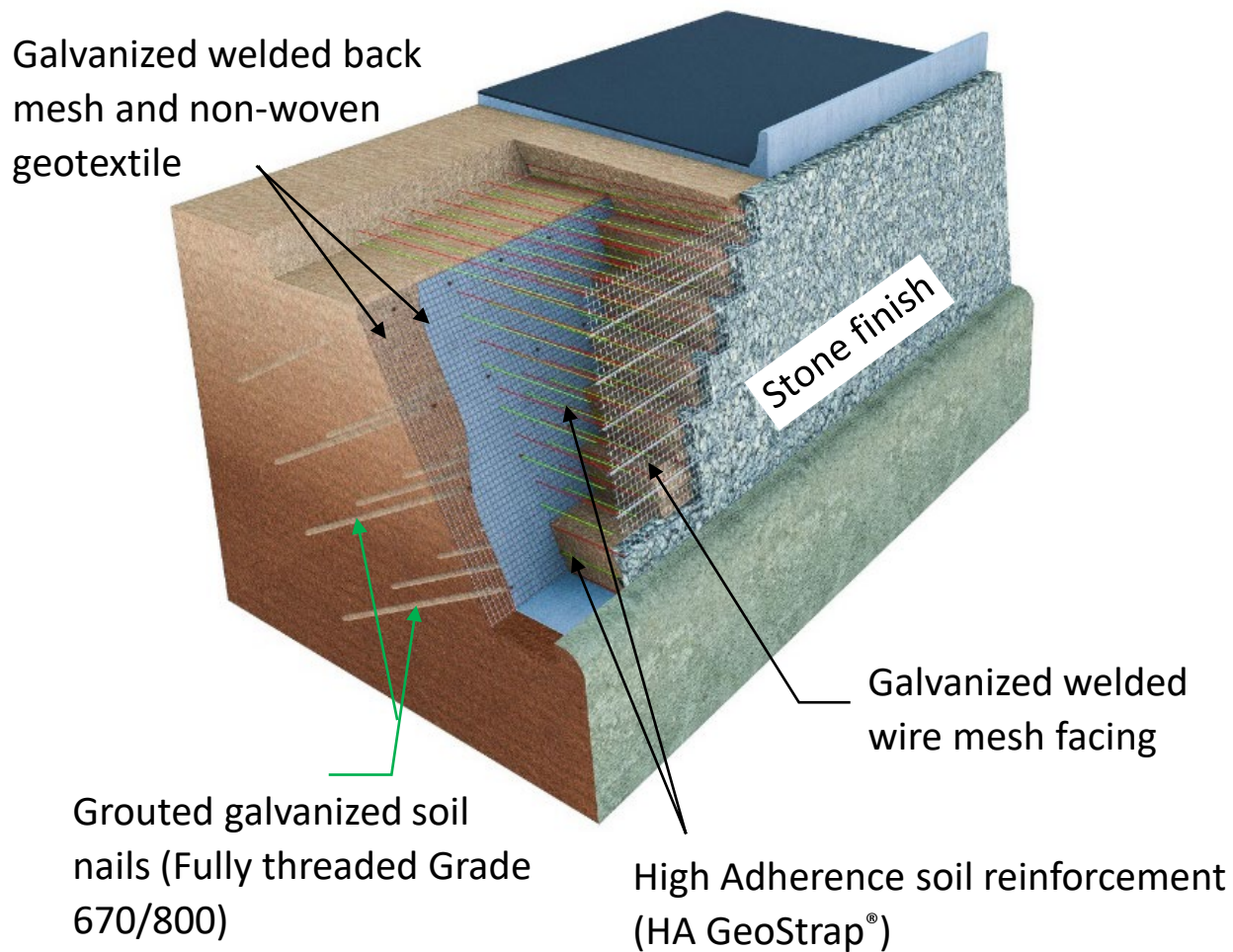
# TerraLink<sup>®</sup> Solution

## TerraLink<sup>™</sup>: a shored Reinforced Earth<sup>®</sup> wall

TerraLink<sup>™</sup> = a **shored** Reinforced Soil Wall / Slope constructed **in front of** an existing structure, **with narrow space between them.**

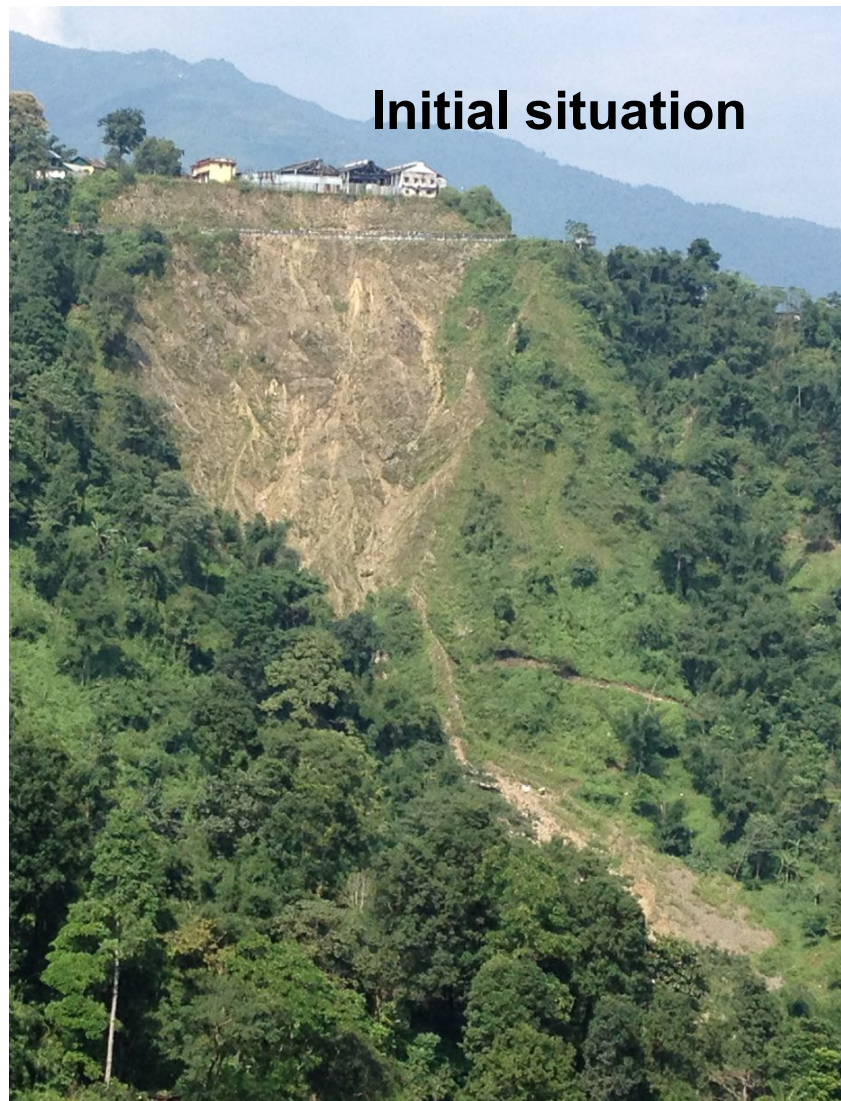


# Solution - Widening through TerraLink®



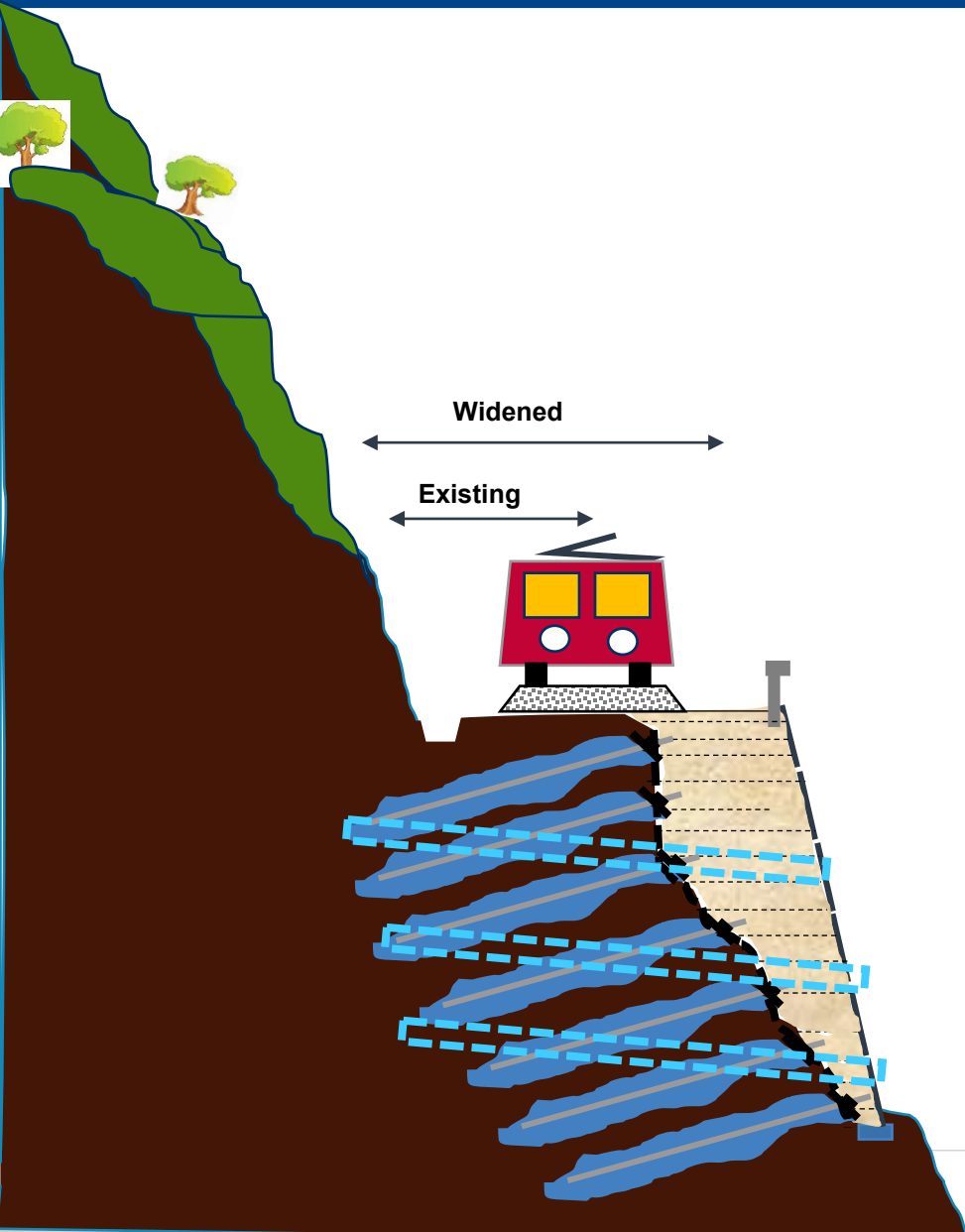


# The Scheme



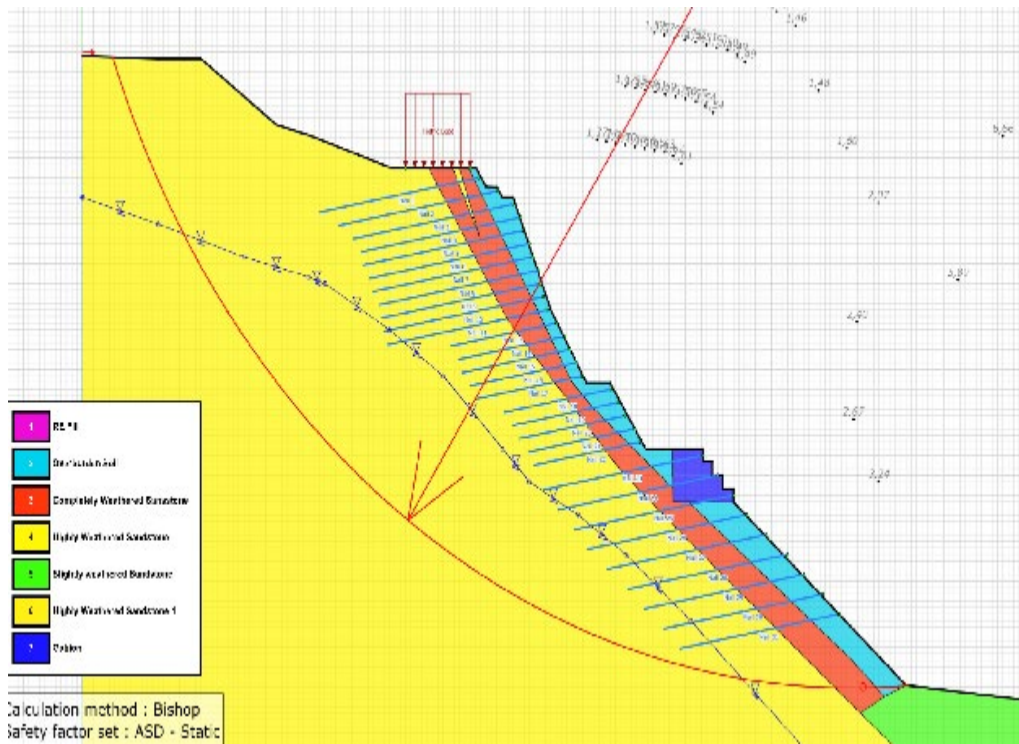


# TerraLink® Installation





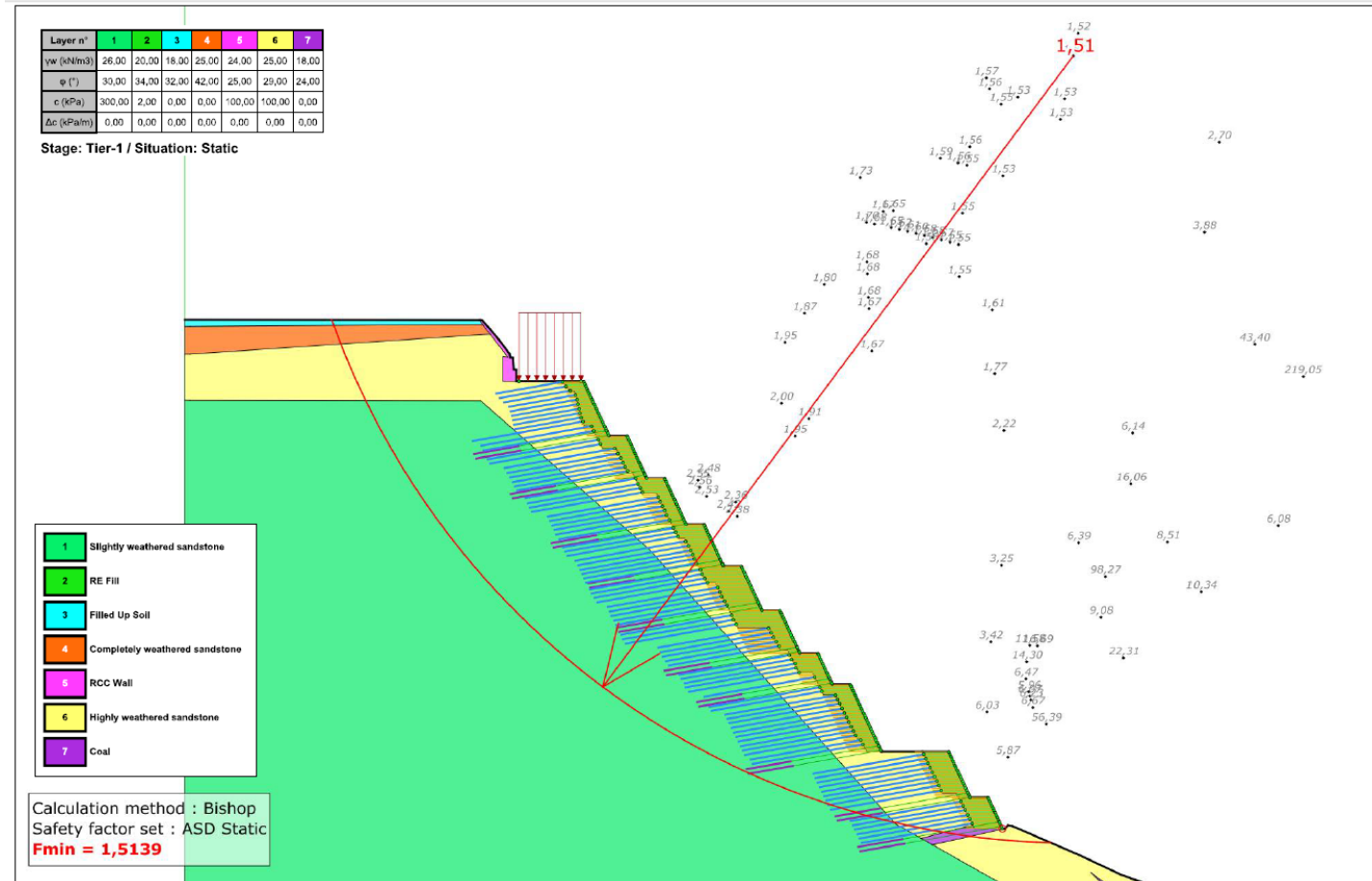
# Design of TerraNail® Structure



Typical output of slip circle analysis along with water profile for static case for soil nailed slope



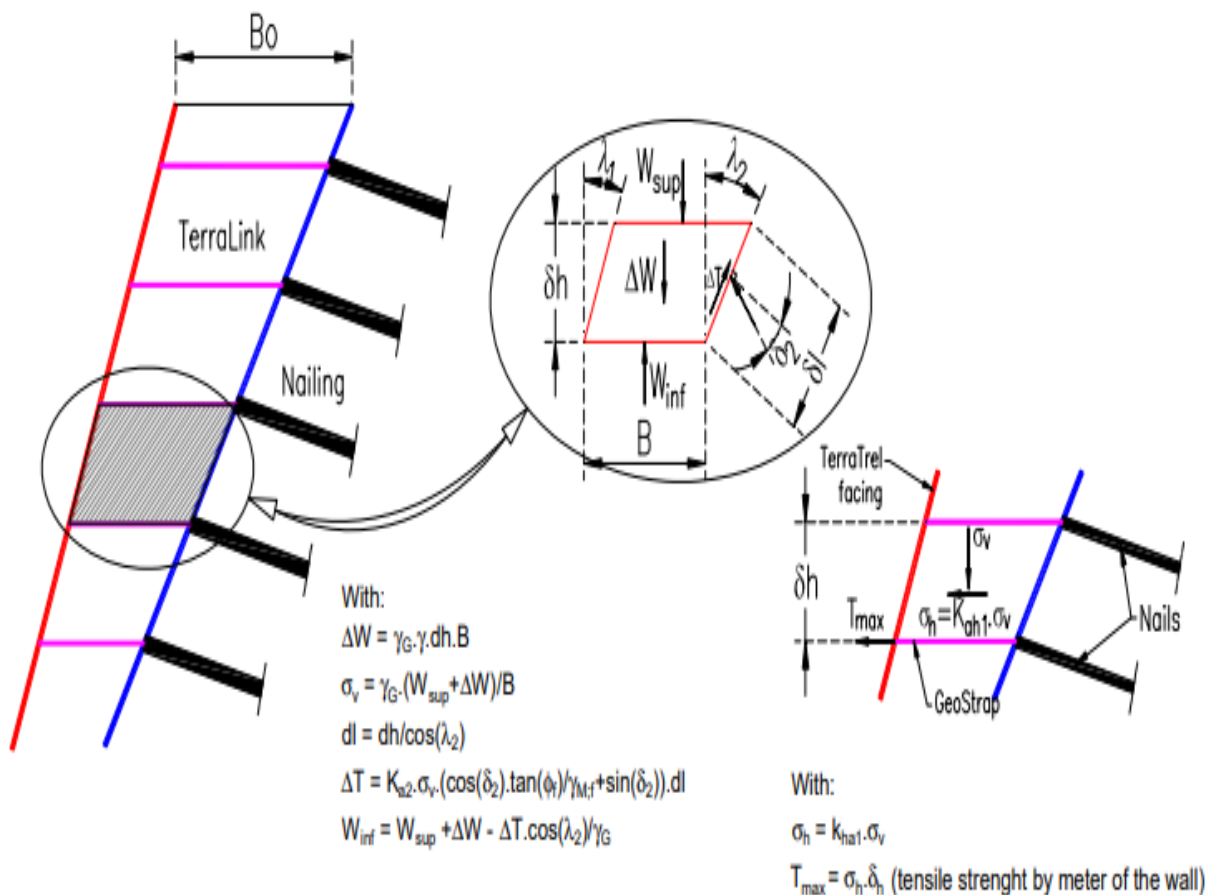
High Adherence (HA) Geosynthetic Strap Soil Reinforcement



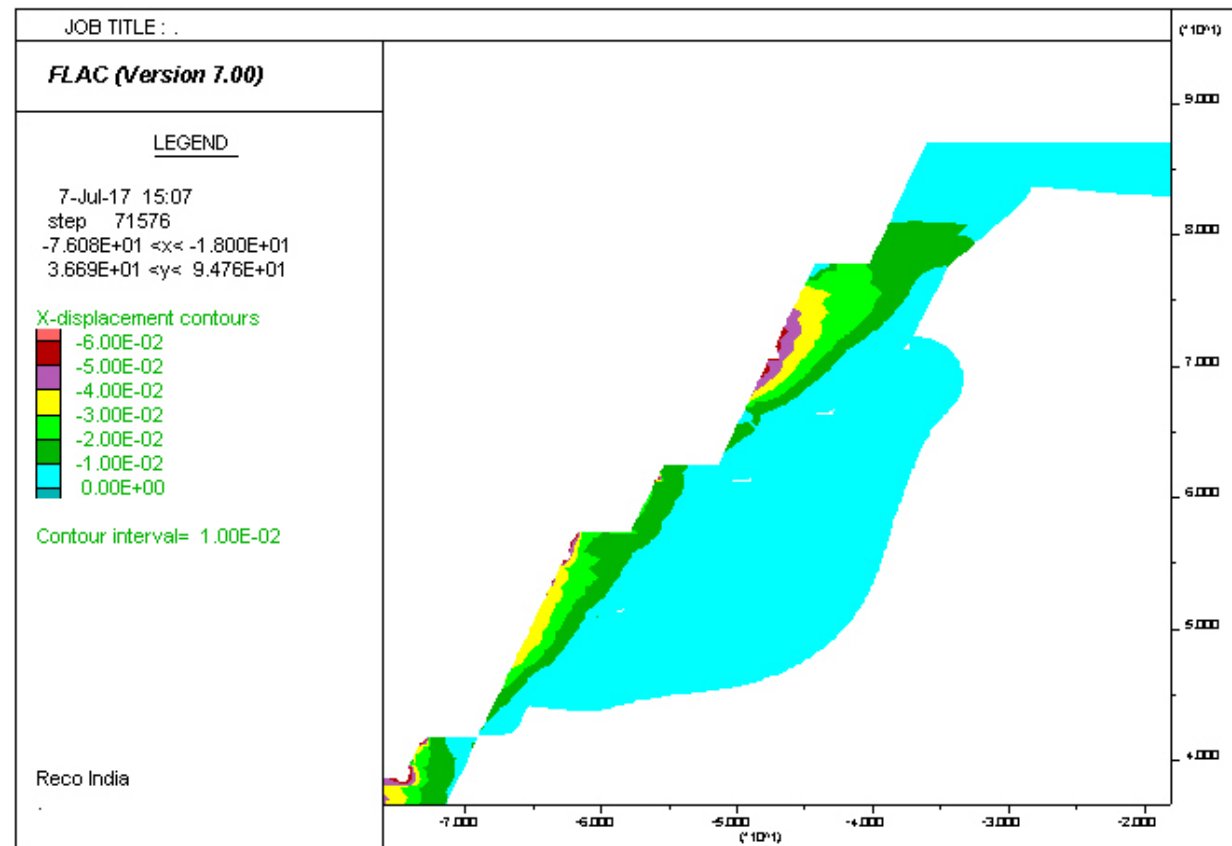
Typical output of slip circle analysis for S2 for Static case for TerraLink® Structure



# Design of TerraLink® Solution

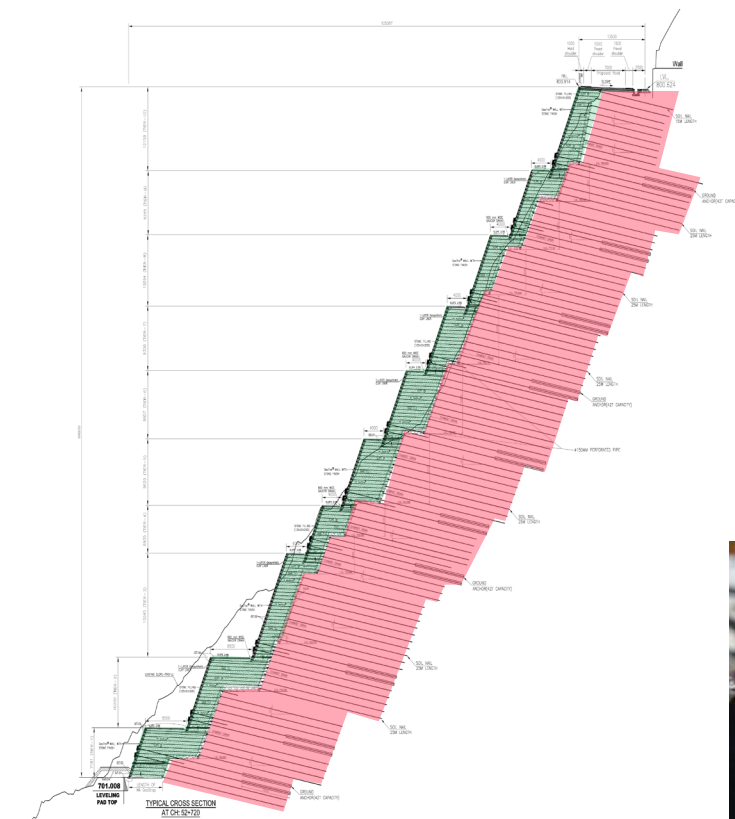


The tension in soil reinforcement by "Silo" calculation considering single friction surface

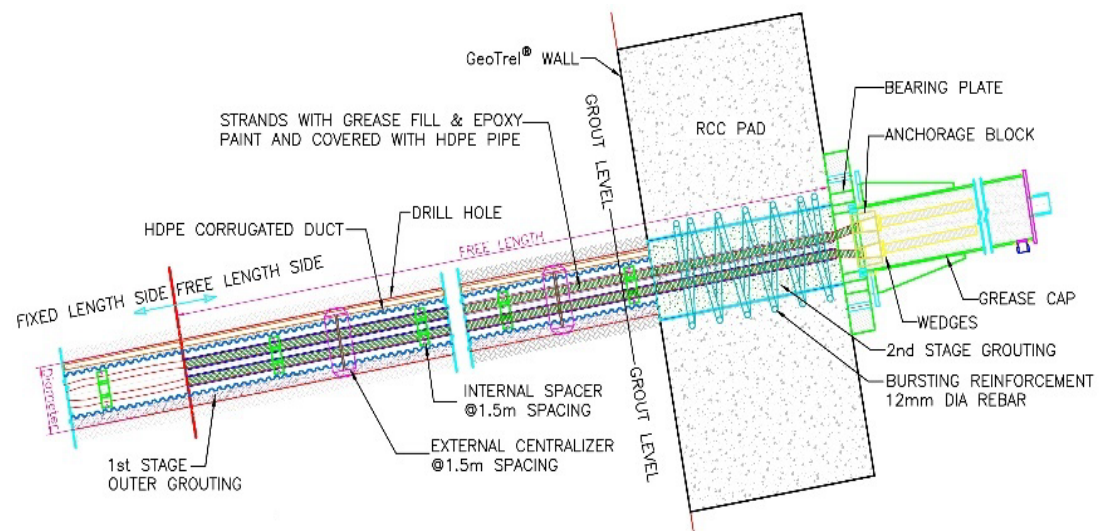
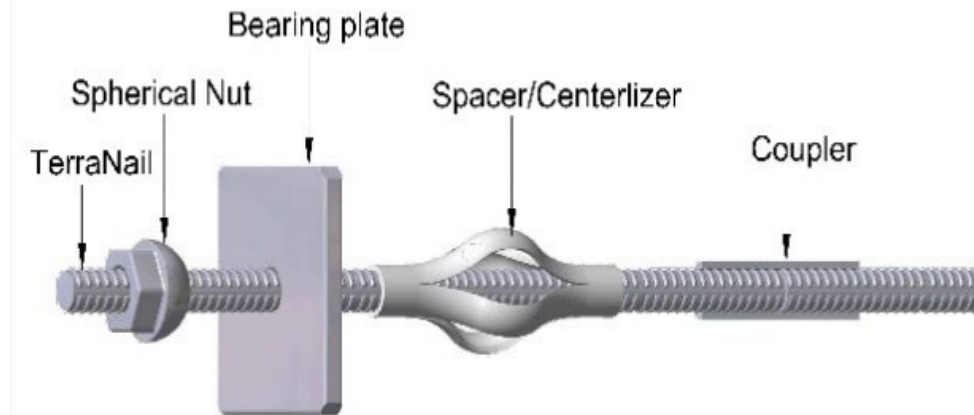
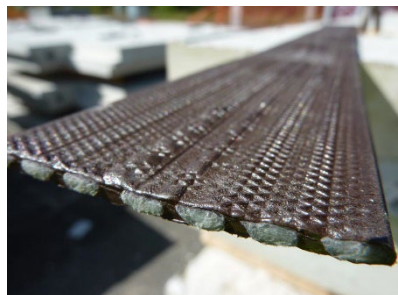


Typical output of FEM analysis

# Approved Design at Location S2



Cross-section



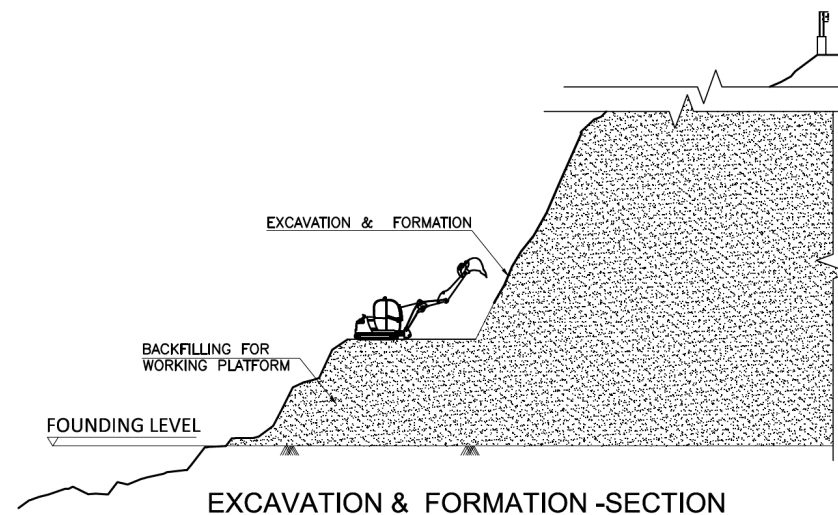
# Execution



**Approach Road**

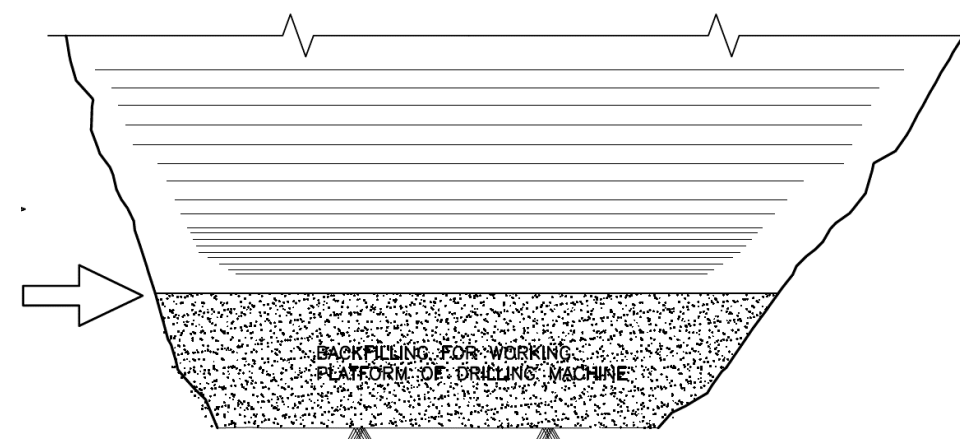


# Excavation – Top to Bottom



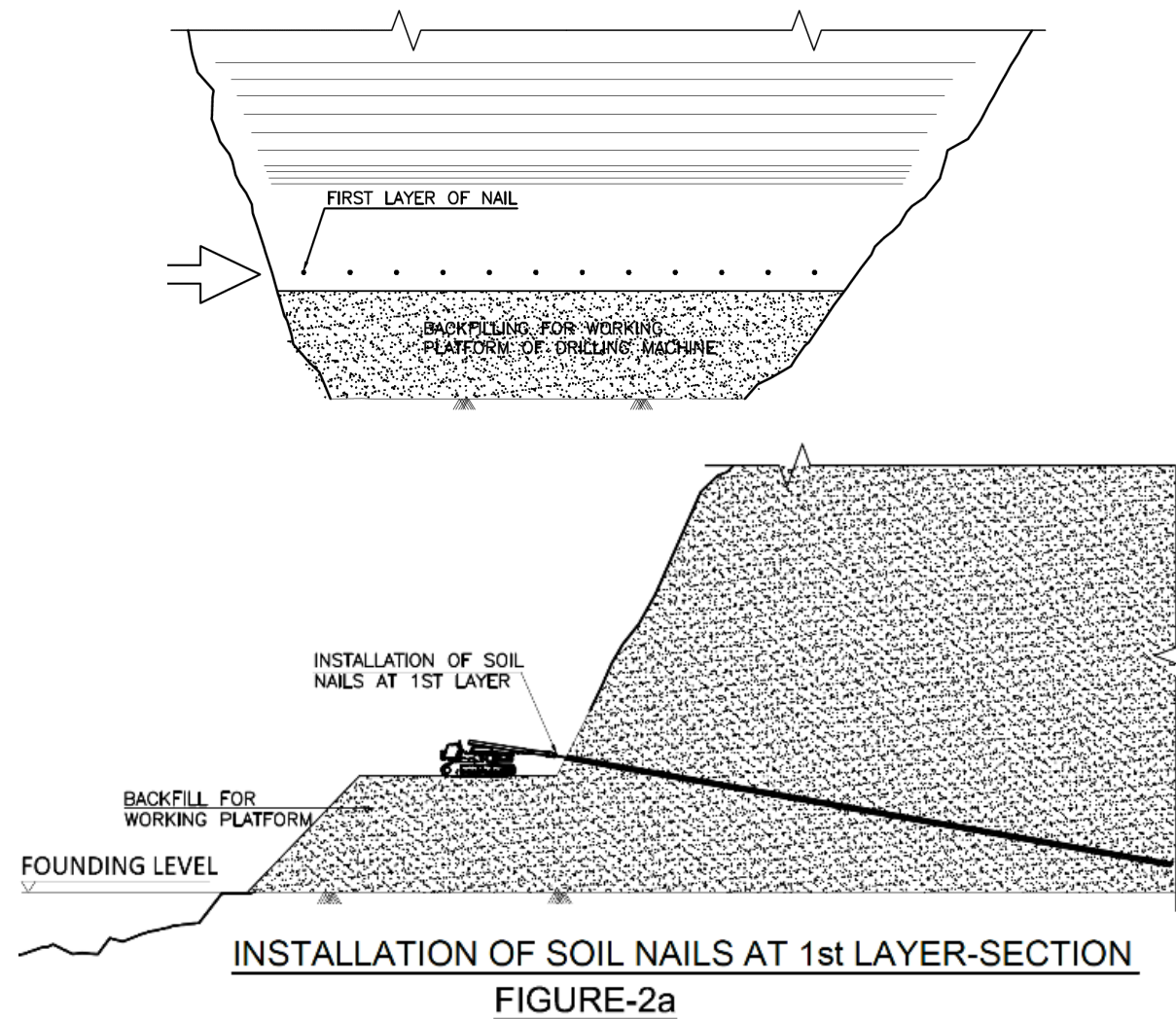
EXCAVATION & FORMATION -SECTION

FIGURE-1a



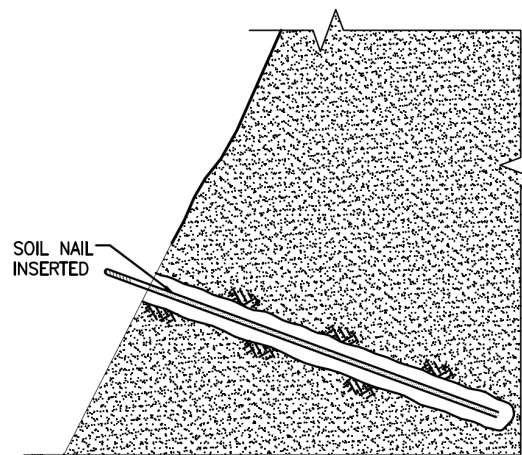


# TerraNail® : Drilling and Installation

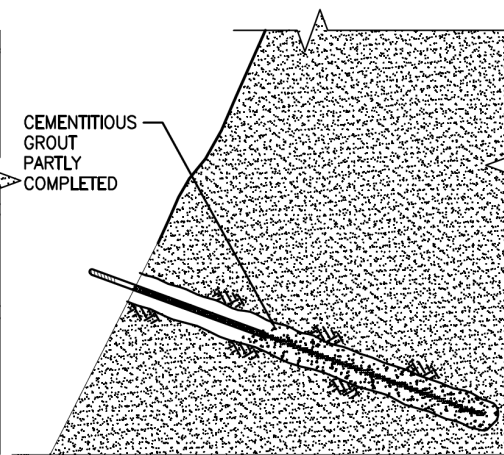




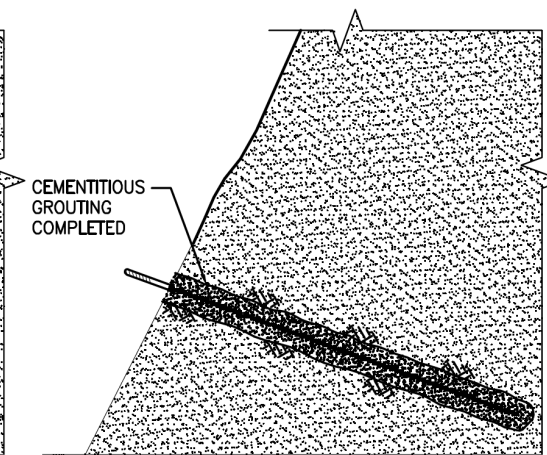
# TerraNail® : Drilling and Installation



GROUTING FIGURE-3a



GROUTING FIGURE-3b



GROUTING FIGURE-3c

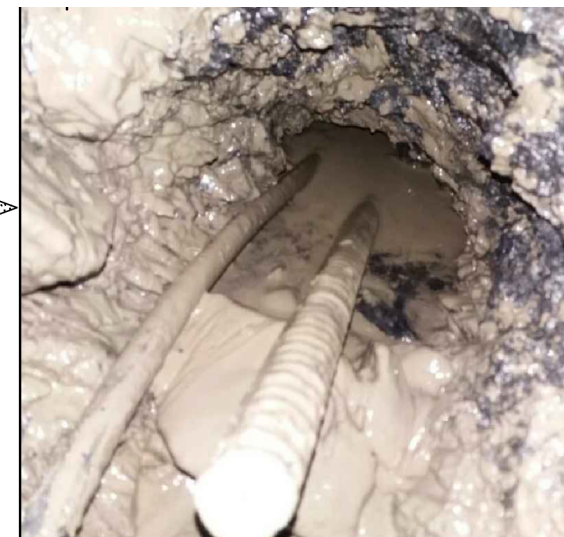
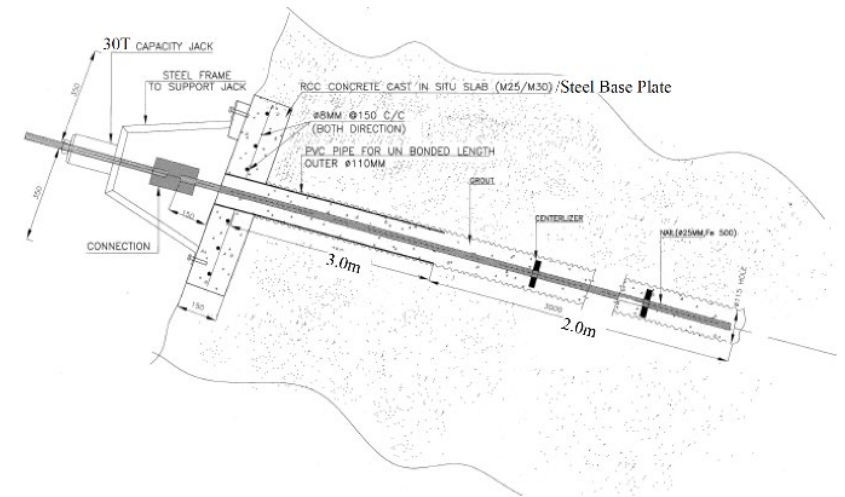


PHOTO 9:-GROUTING





# Pull-out tests



## Pull-out Tests

















































08.04.2019 13:48



06.05.2019 13:5











Completed Structure





**Completed Structure**



# Picture before and after installation

Location : Tindharia



Before



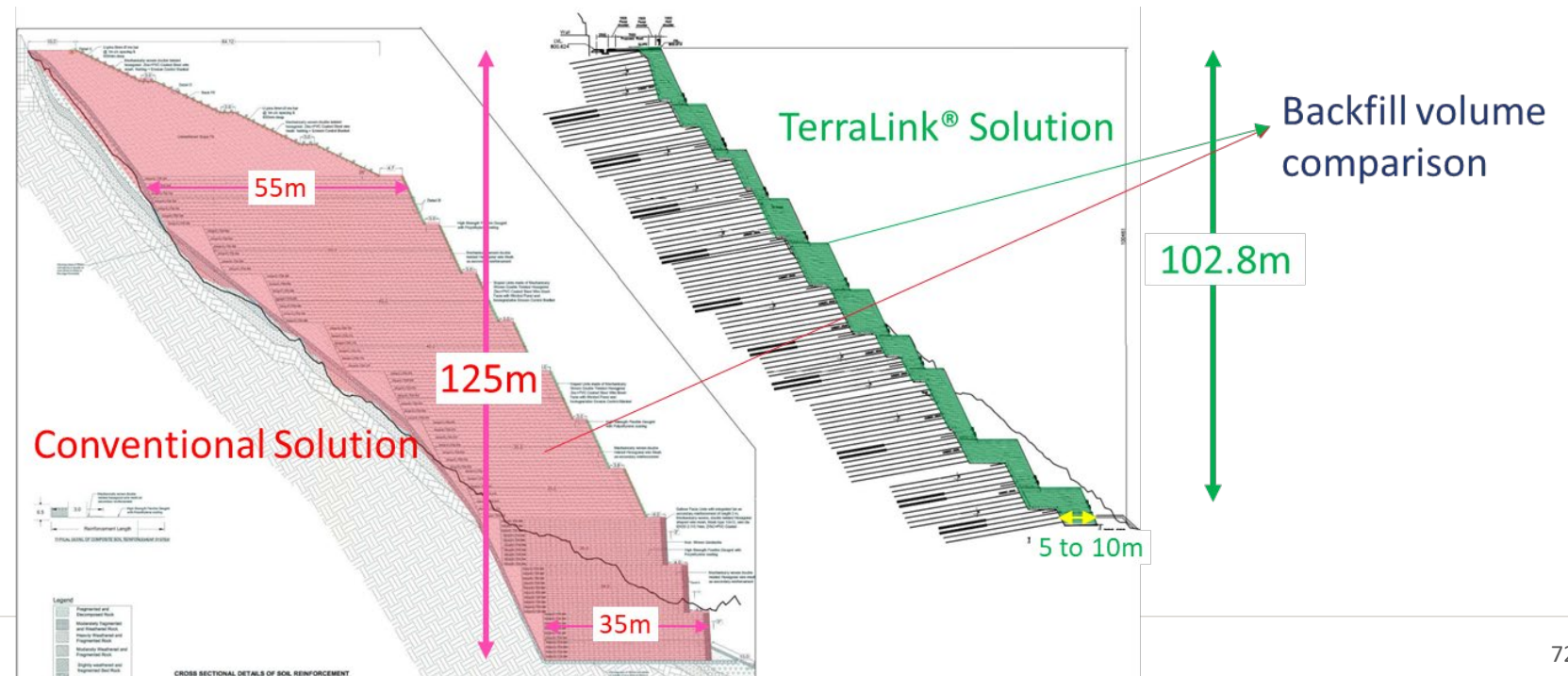
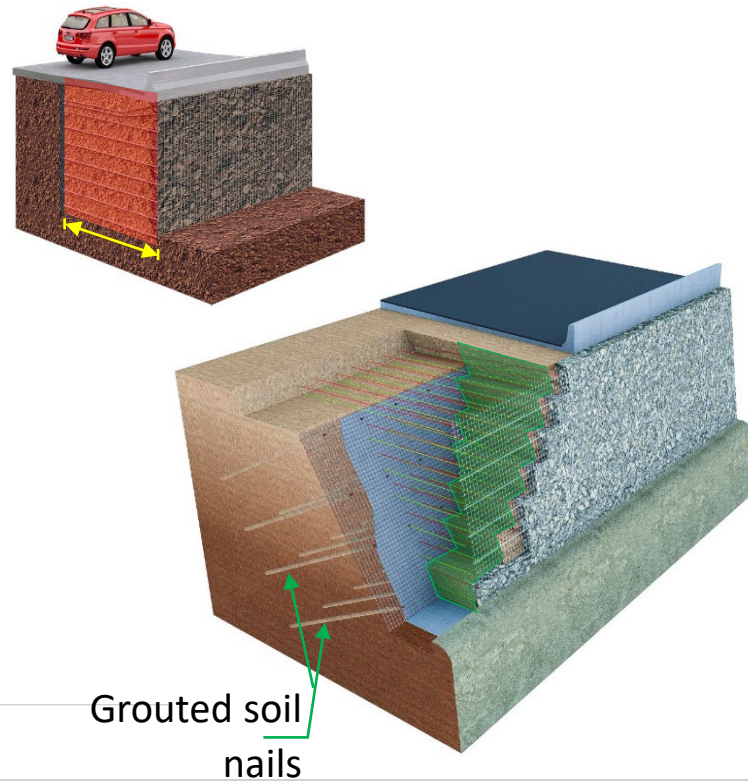
After



# Reduction in Backfill Quantity with TerraLink® Technology

- i) Widening is done using TerraLink® solution where the backfill quantity optimized by 90% (497026 CuM).
- ii) The direct connection system developed by Terre Armée allowed design & construction of such tall retaining structure over a small base width.
- iii) The sub-structure drainage measures adopted by inserting perforated pipes by drilling inside soil mass provided effective drainage system.

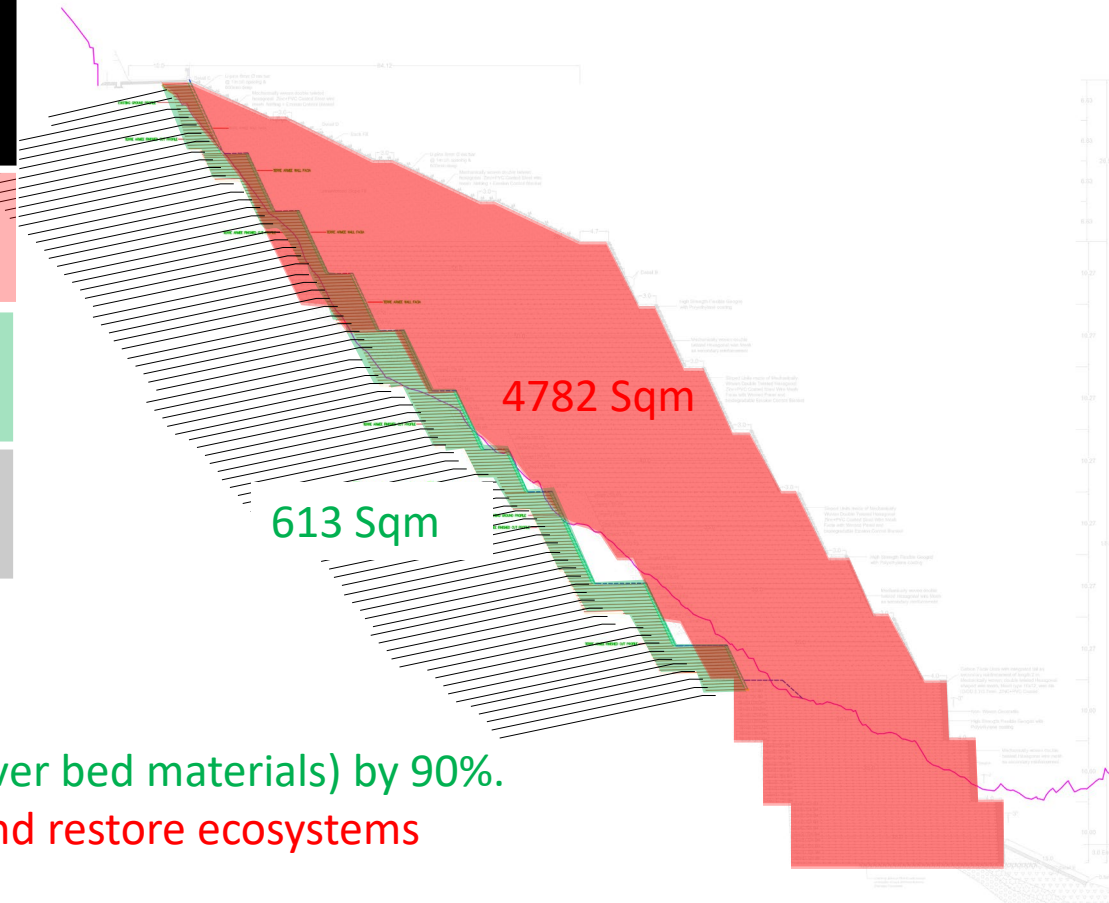
| Conventional Reinforced Soil   | TerraLink® Structure   |
|--|--|
| <ul style="list-style-type: none"> <li>• Gravity retaining structure</li> <li>• Require large base width</li> <li>• Require 10 times backfill soil</li> <li>• More fuel for transportation</li> <li>• More machinery for compaction</li> </ul> | <ul style="list-style-type: none"> <li>• Soil stabilized structure</li> <li>• Very less base width</li> <li>• Less backfill soil</li> <li>• Less fuel for transportation</li> <li>• Less machinery for compaction</li> </ul> |





# Benefits Achieved through backfill reduction

| Description             | Amount of Backfill Required | Transportation Required (No: Trucks x Trips ) | Estimated Cost @ € 31.80 / CuM | Estimated CO <sub>2</sub> Emission (MT) |
|-------------------------|-----------------------------|---|--------------------------------|---|
| Conventional Method     | 557026 CuM                  | 15*37135                                      | € 17.50 million                | 4897                                    |
| TerraLink® Technology   | 60000 CuM                   | 15*4000                                       | € 1.90 million                 | 528                                     |
| Savings (approximately) | 497026 CuM                  | 15*33135                                      | € 15.60 million                | 4369                                    |



- Backfill volume reduction was 90%.
- **Reduction of consumption of natural resources (boulders and river bed materials) by 90%.**
- **Nearby river beds would have been exhausted; hence protect and restore ecosystems**
- Distance of borrow pit to site is around 25 Km
- **Consumption of fossil fuel for transportation reduced by almost 90%.**
- Reduction in emission of polluting gases by almost 90%. **Hence reduce direct greenhouse gas emissions**
- CO<sub>2</sub> emission from earth moving vehicles and compactors was also reduced considerably.





# INITIATIVE for Water Conservation and Recycling

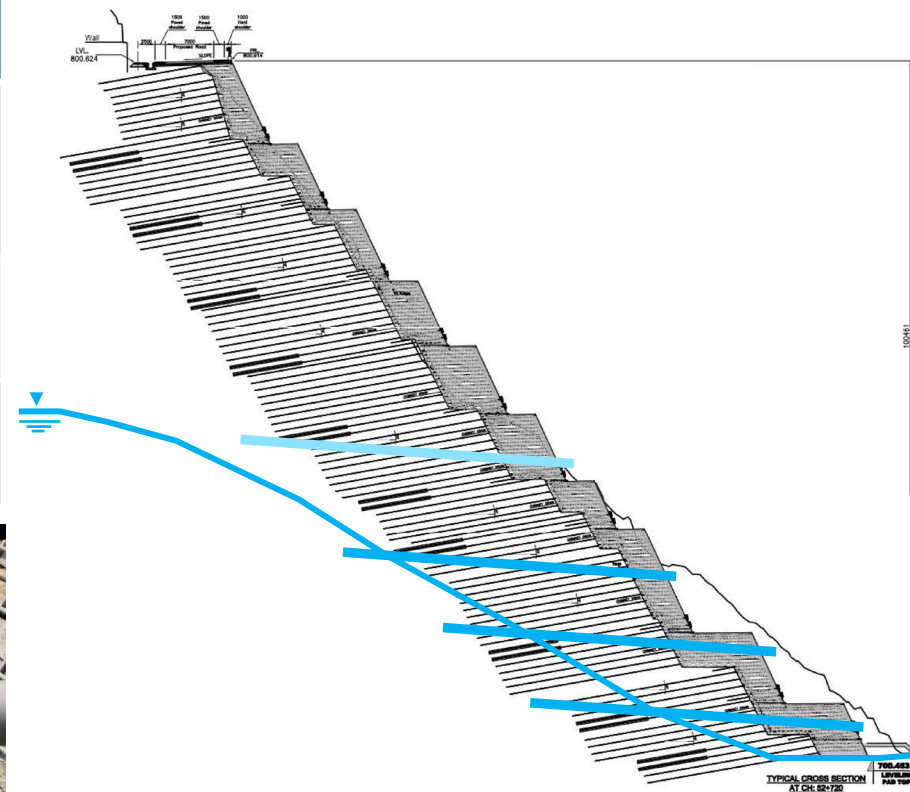
| Normal Practice                     | Innovation implemented with TerraLink®   |
|-------------------------------------|--|
| Short weepholes                     | Large depth weepholes intercepting the sub surface water table - extended outside the TerraLink® Structure |
| Water discharged to suitable outlet | Water collected in the storage tank and consumed locally.  |
| Water not recycled                  | Water recycled for construction and household use.   |



Water outflow during drilling



Filtered water after wrapping Non-woven Geotextile





# Benefits Achieved through Water Conservation

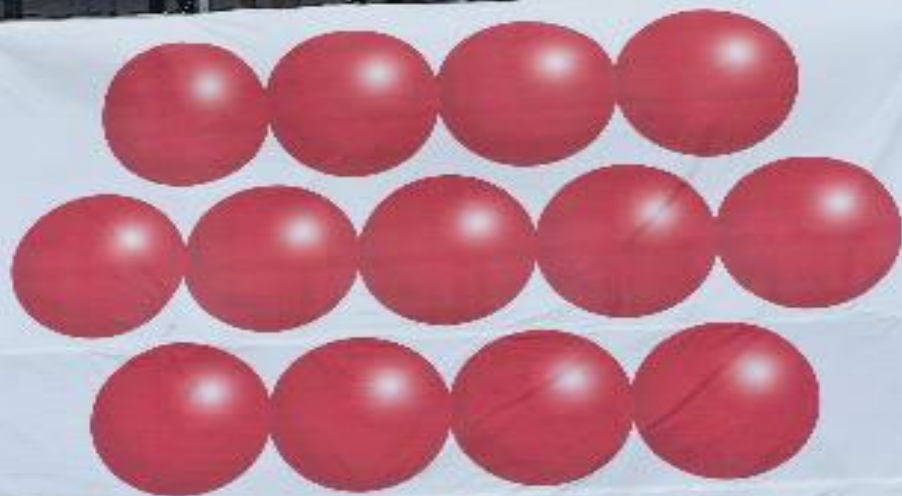
| Description   | Tentative Construction Water Required    | Estimated Cost |
|---|--|----------------|
| <b>Outsourced supply</b>                            | 10 million Litres                        | €55000         |
| <b>After Recycling through weepholes</b>            | 6 million litres                         | €33000         |
| <b>Total Water &amp; Cost Saved (approximately)</b> | 4 million litres                         | €22000         |
| <b>No. of Houses benefitted</b>                     | <b>30 Households (continuous supply)</b> |                |

- Saving **40% of water** helped to protect and restore our ecosystems
- Recycling of discharge water **reduced wastage of water.**
- Water channelized outside the TerraLink® structure was **collected in a storage tank for local consumption.**
- The collected water **was recycled for construction and household use.** Residents have been enormously benefited and their water crisis has been permanently resolved.
- Structure stability improved.

Large depth weepholes in the form of semi-perforated pipes wrapped with non-woven geotextile and extending into the existing ground profile were provided to intercept & channelize sub-surface water table outside the TerraLink® Structure.







**TERRE ARmee**